807/D-32

SUBSURFACE INVESTIGATION
MONUMENT GROUNDS
AND
VISITOR FACILITY
WASHINGTON MONUMENT
WASHINGTON, DC

National Park Service National Capital Parks - Central 900 Ohio Drive, SW Washington, DC 20024-2000

Olin Partnership
The Public Ledger Building
150 South Independence Mall West
Philadelphia, PA 19106

Hartman Cox Architects 1074 Thomas Jefferson Street, NW Washington, DC 20007

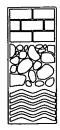
Mueser Rutledge Consulting Engineers 225 West 34th Street New York, NY 10122

June 10, 2002

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Tel: (917) 339-9300 • Fax: (917) 339-9400

www.mrce.com

George J. Tamaro Peter H. Edinger Alfred H. Brand Hugh S. Lacy David M. Cacoilo Joel Moskowitz Peter W. Deming Partners 1 4 1

June 10, 2002

Elmer A. Richards Edmund M. Burke John W. Fowler J. Patrick Powers Consultants

National Park Service National Capital Parks - Central 900 Ohio Drive, SW Washington, DC 20024-2000

Re:

James L. Kaufman Daniel M. Hahn Raymond I. Poletto Roderic A. Ellman, Jr. Thomas R. Wendel Francis J. Arland Robert M. Semple Theodore Popoff David R. Good

Mr. Arnold Goldstein, Superintendent Attention:

Senior Associates Domenic D'Argenzio Walter E. Kaeck

Subsurface Investigation

Washington Monument

Robert K. Radske Harro R. Streidt Ketan H. Trivedi Michael J. Chow Alice Arana Douglas W. Christie Monument Grounds and Visitor Facility

MRCE File 9726

Joseph N. Courtade Director of Finance and Administration

Hiren J. Shah

Associates

Gentlemen:

In accordance with our proposals dated March 13, 2002, we summarize herein the results of our soils and foundation investigation for the Monument Grounds and Visitor Facility at the Washington Monument.

Martha J. Huguet Marketing Manager

Exhibits

The following exhibits are attached to illustrate our report:

Drawing No. B-1 Boring Location Plan Geologic Section A-A Drawing No. GS-1 Geologic Sections B-B and C-C Drawing No. GS-2 Geologic Sections D-D and E-E Drawing No. GS-3 Geologic Sections F-F and G-G Drawing No. GS-4 Drawing No. GS-R Geotechnical Reference Standards Laboratory Data Summary Table No. 1

Appendix A Boring Logs

Appendix B Consolidation Test Data - MRCE

Consolidation Test Data by Barber - MRCE Appendix C

Interpretation

Appendix D 1984 Report by MRCE 1973 Report by MRCE Appendix E

Available Information

The following items used in the preparation of our report were obtained from Hartman Cox Architects and Olin Partnership:

- 1. A topographic survey of the site dated March 28, 2002, prepared by Wiles Mensch.
- 2. A drawing of the site indicating the proposed grading and landscape walls.

The following items used in the preparation of our report were obtained from our files:

- 1. A report entitled Subsurface Investigation and Foundation Study for Interpretative Facility at the Washington Monument, Washington, DC, National Capital Parks, dated September 14, 1973, prepared by MRCE for the National Park Service.
- 2. A report entitled Settlement Analysis for Washington Monument Grounds Improvement, NPS: D18 (NCR-NACC) (XF54), dated February 13, 1984, prepared by MRCE for the National Park Service.
- 3. A report entitled Loading Limitations, Washington Monument Grounds, Washington, D.C., 1962," dated December 31, 1962, prepared by Edward S. Barber, Consulting Engineer, for the Department of the Interior.
- 4. Logs of borings made in 1930 for a study of the Washington Monument.

Site Description

The Washington Monument is located on a grassy knoll on the National Mall between Constitution and Independence Avenues, between 15th and 17th Streets. The grade around the Monument is approximately Elev. 40 referenced to Mean Sea Level.

Site History

The history of the construction of the Monument is well-documented in a number of works and there is no need to repeat it here. In brief, the foundations were constructed in 1848 and the shaft was begun at the end of 1848. Construction halted in 1854 and resumed in 1878 with underpinning of the original foundations. The underpinning was carried to about Elev. +2. The remainder of the Monument was constructed between 1878 and 1884. Settlement has been monitored throughout its history, but available records date back to 1878. They indicate that total settlement between 1879 and 1992 was about 7 inches, due to the compression of the T1(D) clay. During the 7-year completion of the Monument, 4.5 inches of this settlement occurred. During the subsequent 106 years (1886-1992) settlement has been less than 2.5 inches.

Project Description

The Visitor Facility will consist of a below-grade structure constructed immediately west of the Monument Lodge. It will occupy a footprint area of about 15,000 square feet, with the floor at about Elev. 5.5. Subgrade is expected to be at Elev. +3. An elevator and stairway entrance will be constructed within or adjacent to the Monument Lodge. An underground passageway will be constructed from the Visitor Facility to the Monument, and will enter the Monument's east side below grade.

The planned vehicle barrier system involves a series of low walls, many bordering a new system of paved walkways in combination with a modest regrading over an extensive area around the Monument. The planned walls and paved walkways are shown in bold on Drawing No. B-1 along with existing contours of ground surface elevation. In general, grades tend to be raised one foot or less within 150 feet of the Monument, and raised one to two feet within 150 to 200 feet from the Monument. Fills of two to five feet are planned beyond 200 feet from the Monument on the north, west and southeast, and cuts of up to three feet are planned southwest and northeast of the Monument. The regrading was planned taking into consideration the load change limitations at varying distances from the Monument described in the 1973 report.

Field Exploration

Field work began on February 28, 2002 and finished on March 15, 2002. Six borings were made at the proposed location of the Visitor Facility and one at the Monument Lodge. These borings are identified with a B prefix. Twenty borings were made for the vehicle barrier system, and are identified with a W prefix.

Borings were made by GeoServices Corporation of Forestville, MD, under contract to Grunley-Walsh Construction, JV. All field work was performed under the inspection of our resident engineers, Mr. Michael Weckler and Mr. Oscar Carpio. Boring locations and elevations were provided by the boring contractor's surveyor. Elevations are referenced to the National Geodetic Survey Datum, a mean sea level datum.

Representative soil samples were recovered from the borings with a two-inch split spoon sampler driven with a 140-pound hammer free-falling 30 inches. The number of blows required to advance the sampler through each of three or four six-inch intervals was recorded. The Standard Penetration Test (SPT) resistance, an index of the density of the material sampled, is calculated by summing the blows from the second and third intervals.

B-series borings were advanced and stabilized using weighted drilling fluid and temporary casing, and extended to depths of 50 feet except those in which decomposed rock was sampled, which extended to depths of 85.5 and 89.4 feet. Relatively undisturbed samples of potentially compressible deposits were recovered in two of the borings using three-inch tubes advanced with

a Shelby-type sampler. Piezometers were installed in two of the completed borings to determine present groundwater levels.

W-series borings were advanced using hollow-stem augers. All borings except two extended to depths of 12 feet below existing grade. One boring encountered obstructions and was terminated at five feet and one extended to 16 feet. Relatively undisturbed samples of potentially compressible soils were recovered in four of the borings using three-inch tubes advanced with a Shelby-type sampler.

After completion of the borings, GeoServices Corporation shipped the soil samples to our office. Samples were reexamined in our laboratory and field descriptions were verified or revised as necessary. All soil samples are described in accordance with the system shown on Drawing No. GS-R. Relatively undisturbed samples were subjected to laboratory testing to determine strength and consolidation characteristics. Groundwater levels were recorded during the course of the field work. Readings are shown on the appropriate boring log sheets.

SUBSURFACE CONDITIONS

The results of the boring program are shown on the boring logs attached as Appendix A. The logs include sample number, depth, blow count, individual soil descriptions for each sample and descriptions of drilling operations. Our interpretation of subsurface conditions is illustrated on Geologic Sections A-A through G-G, shown on Drawings Nos. GS-1 through GS-4. Generalized descriptions of the soil strata encountered in the borings are summarized below in order of their occurrence with increasing depth:

Stratum F - Fill

The uppermost material encountered in all of the recent borings is fill ranging in thickness from 2 to greater than 16 feet. Stratum F consists of loose to very compact brown silty fine to medium sand grading to fine to coarse sand, some silt with fine sandy silt, trace to some gravel, trace brick, concrete, clay, vegetation shells.

Stratum T1(A) - Sandy Silt

This stratum was encountered beneath Stratum F in 20 of the recent borings and beneath Stratum T2 in one boring. Measured thicknesses ranged from 2 to 17.7 feet, but the stratum extended beyond the bottom of some of the W-series borings. Stratum T2 was interlensed with Stratum T1(A) in four of the recent borings. Stratum T1(A) consists of loose to medium compact brown fine sandy silt, trace clay, clay pockets, gravel, lignite or stiff brown clayey silt to silty clay, trace to some fine sand, trace gravel, lensed with silty fine sand, and fine sandy clay.

Stratum T2 - Silty Sand

Stratum T2 was encountered beneath Stratum F or T1(A) in nine of the recent borings and extended beyond the bottom of some of the W-series borings. Stratum T2 was also interlensed with Stratum T1(A) in four of the recent borings and lenses ranged in thickness from 2 to 3 feet. Stratum T2 consists of loose to medium compact brown silty fine to medium sand, trace clay, gravel, grading to fine to medium sand, some silt, trace clay, gravel.

Stratum T3 - Sand and Gravel

Stratum T3 was encountered beneath Strata T1(A) and T2 in all B-series borings and ranged in thickness from 17.5 to 24 feet. Stratum T3 consists of compact to very compact brown fine to coarse sandy gravel, trace to some silt, grading to gravelly fine to coarse sand, some silt, with occasional boulders and cobbles.

Stratum T1(D) - Plastic Clay

Stratum T1(D) was encountered beneath Stratum T3 in all B-series borings and ranged up to 43.5 feet in thickness. Stratum T1(D) consists of soft to stiff gray plastic clay to silty clay, trace to some fine sand, trace fine sand layers and pockets, gravel.

Stratum D - Decomposed Rock

Stratum D was encountered in the two B-series borings that extended deeper than 50 feet, at a depths of about 85 feet. Stratum D consists of very compact gray micaceous fine to medium sand, some silt, trace to some rock fragments.

Groundwater

Groundwater was measured in two permanent piezometers installed during the field work. Groundwater levels corresponded to Elev. -5.7. Subsequent readings have shown little change in water level.

LABORATORY TESTING

We performed consolidation testing on three relatively undisturbed samples of T1(D) clay in our laboratory. We performed strength tests on five relatively undisturbed samples of the T1(A) clay and five relatively undisturbed samples of the T1(D) clay. Results are summarized on Table No. 1 and presented in Appendix B.

We analyzed and plotted the results of 44 consolidation tests performed by Edward S. Barber in 1962. Results are presented in Appendix C.

CONCLUSIONS

Based on the results of our field and laboratory programs, we conclude the following:

- 1. Soil stratigraphy is as presented in earlier reports.
- 2. Water is at approximately Elev. -6.
- 3. As deepest excavations are expected to be to about Elev. +3, no dewatering will be required to construct the proposed facilities.
- 4. As no dewatering is anticipated, there will be no drying of Stratum T1(D), the clay layer which indirectly supports the Monument.
- 5. Stratum T1(A), the upper clay layer, does not extend beneath the Monument and is above the water table. Thus it may not be fully saturated and if kept dry construction should not disturb its strength.
- 6. The Visitor Facility walls and columns will bear on soils of Strata T2 and T3. Allowable bearing pressures will be on the order of 2 to 3 TSF on Strata T2 and T3. Support of the floor of this structure on Stratum T1(A) will be considered during final design. We anticipate that the new loads imposed by the building foundations will generally not exceed the weight of the soil removed.
- 7. The underground passageway from the Visitor Facility to the Monument will bear on Strata F, T1(A) and T2. Allowable bearing pressures will be on the order of 1 to 1.5 TSF in Stratum F, and 1.5 to 2.5 TSF on Strata T1(A) and T2. We anticipate that the new loads imposed by the passageway will not exceed the weight of the soil removed. Given the varying depth of construction, the weight of soil removed ranges from about 0.7 to 1.4 TSF.
- 8. Braced below-grade walls will be entirely above the water table. They should be designed for lateral pressures of 75 psf per foot of depth below grade plus surcharge loading. Braced walls would include those for the Visitor Center and tunnel. Roof slabs should be designed for 130 pcf per foot of cover, plus the weight of the slab. A surcharge live load should be added to the above design numbers in the event that maintenance vehicles are operated near or above the structure, or that a large event on the Mall could cause crowds to gather near or above the structure.
- 9. It is our understanding that the Monument Lodge is supported on spread footings bearing about three feet below current grade. This places the footings on Stratum T1(A) soils. Excavation of a full or partial basement to permit construction of the escalator or elevator for entrance to the security facility will require underpinning some or all of the building.

All excavations would be within Strata T1(A) or T3 and above the water table. Underpinning should extend to and bear on Stratum T3. Allowable bearing pressure for underpinning and new footings on Stratum T3 is 3 TSF.

- 10. Landscape barrier walls will be founded in soils of Strata F, T1(A), and possibly T2. Allowable bearing pressures below these walls should be limited to 1 TSF. Lateral design pressure will vary with slope of the ground behind the wall from 50 psf per foot of depth for a horizontal surface to 65 psf per foot of depth for a slope of 20 degrees above horizontal. A surcharge live load should be added to the above design numbers in the event that maintenance vehicles are operated near or above the walls, or that a large event on the Mall could cause crowds to gather near or above the walls.
- 11. Our laboratory testing confirmed consolidation parameters used in evaluating settlement in our previous reports.
- 12. After discounting those tests for which Barber's consolidation test results were irregular, we found good agreement between Barber's results and ours.
- 13. The results of the strength testing confirmed the shear strength values presented in our 1973 report.

RECOMMENDATIONS

The following recommendations are provided for use in designing the landscape grading at the Washington Monument. As described in the Environmental Assessment (EA) for the project, the cut and fill of soils on the Grounds would be designed to result in no significant change in the soil loads beneath the Monument.

After obtaining new field samples and performing laboratory tests on the soils, and comparing the results of these tests to the data in the 1973 and 1984 we found no changed conditions in the soils that would alter the recommendations made in 1973.

Therefore, the recommendations from MRCE's 1973 report are still valid and are restated below. Note that these recommendations were prepared for a proposed Visitor Facility.

1. Restrictions must be placed on the net permanent change in loads within a distance of several hundred feet from the center of the Monument. More severe limitations would apply to asymmetrical loads than to loads placed symmetrically on all sides of the Monument since it is the one-sided loading which will produce a tilt of the shaft. More stringent limitations should be applied to the addition of load compared to a reduction of load since an increase in stress beneath the Monument foundation will produce settlements in the range of virgin compression. A permanent net increase in loading

within 150 feet from the monument center should generally not exceed 1,000 pounds per square foot. If such an additional load were to be placed over a sector of 180 degrees on one side of the foundation it would produce a tilt of the top of the shaft of nearly one inch and detailed studies if its specific effect would be desirable.

- 2. The net change in load equals the total dead load plus permanent live load applied by the structure plus any filling or backfilling minus the total weight of excavated soil removed by excavation for the structure. For purposes of appraising the magnitude of loading, the unit weight of excavated soil can be assumed at 130 pounds per cu. ft. If construction of an underground facility produces a net reduction in loading, the average magnitude of this reduced loading should be limited to about 1500 psf asymmetrically or 2000 psf if it is positioned symmetrically around the Monument within a distance of 150 feet from the Monument center. The precise effect of a specific asymmetric loading in producing tilt of the shaft would have to be evaluated in detail when a final scheme is evolved.
- 3. Permanent net changes in load taking place within 63 feet of the Monument center, that is, overlying the projected base of the underpinned foundation, generally should not exceed 500 psf and should be restricted in lateral extent to the least practicable dimensions. For a structure of small dimension such as a passageway or narrow corridor, this 500 psf limit might be relaxed after specific study. [Note: Anticipated loads in the current 2002 design do not approach 500 psf.] The magnitude of pressures which are transmitted to soil directly beneath the Monument foundation from wall footings column footings, slabs or other foundation units of the newly constructed facility must be strictly limited. We believe that this pressure increase should not exceed about 1 to 1.5 kips per sq. Ft. The effects of a local or asymmetric increase in pressure beneath the Monument foundation would have to be evaluated in detail for the specific scheme of new construction.
- 4. Beyond 150 feet from the Monument center permanent net changes in load will have little if any effect on the foundation performance of the Monument but net changes exceeding 1500 psf additive load or 2000 psf net reduction in load with lateral dimensions of more than 150 feet asymmetric to the Monument should be kept beyond 200 feet from the Monument center if possible.
- 5. Excavations for construction of the Interpretive Facility [Note: There was no security element considered in 1973] should be limited as follows to avoid any possible disturbance to the soils that are carrying the loads of the Monument.
 - a. Within 115 feet from the Monument center no excavation for construction of the Interpretive Facility should be allowed to extend below Elev. +16, that is, the level of the base of the original stone foundation. The width of such excavations before placing of structure concrete and partial backfilling should not exceed 45 feet.

- b. Beyond 150 feet from the Monument center temporary excavations for construction can be carried down to as low as Elev. 0 provided the excavation and replacement by construction is carried out in width increments not exceeding about 100 feet.
- c. Between 115 feet and 150 feet from the Monument center the maximum depth of temporary excavation is defined by a line sloping downward from Elev. +16 with a slope of 1 vertical on 2.6 horizontal and width limitations are intermediate between those stated in a and b.
- d. Excavation or a widespread structure symmetrically placed which would approach the limitation on maximum load removal must be carried out with great caution. Specifications should require a program of excavation in which load removal on opposite sides of the Monument would be reasonably well balanced at all stages of the operation.
- e. In general, it would be preferable to stabilize the sides of excavations near the Monument by cutting on sloped banks rather than by driving sheet piling of soldier piles for a cofferdam. Where vertical-wall cofferdams are absolutely necessary these could be formed by soldier piles placed in pre-augered holes.

MRCE was asked to restate these recommendations in reference to the current designs and in a tabular format which makes it easy to determine what loadings are recommended within each distance range from the Monument center. This results in the following recommendations:

ALLOWABLE LOADING

Distance from	Allowable	Allowable	Remarks
Monument	permanent net	permanent net	
center	increase	decrease	
up to 63 feet	500 psf	500 psf	Minimize lateral extent 500 psf may be relaxed for small footprint after study
63 to 150 feet	1000 psf	1500 psf asymmetrically	
		2000 psf symmetrically	
150 to 200 feet	1500 psf	2000 psf	limits for asymmetric loading with lateral dimensions of more than 150 feet
200 feet or more	unspecified	unspecified	

Loading is subject to analysis in every case to determine its effects on the subsoils.

ALLOWABLE EXCAVATION

Distance from Monument center	Allowable excavation	Remarks
up to 115 feet	No deeper than Elev. 16	Maximum width open at any time is 45 feet
115 to 150 feet	Following a line from Elev. 16 at 1V:2.6H	Maximum width open at any time is between 45 and about 100 feet, proportional to distance from Monument center
150 feet or more	No deeper than Elev. 0	Maximum width open at any time is about 100 feet

Excavation is subject to notes (d) and (e) of Item 5 above.

Please contact us if you have any questions concerning this report.

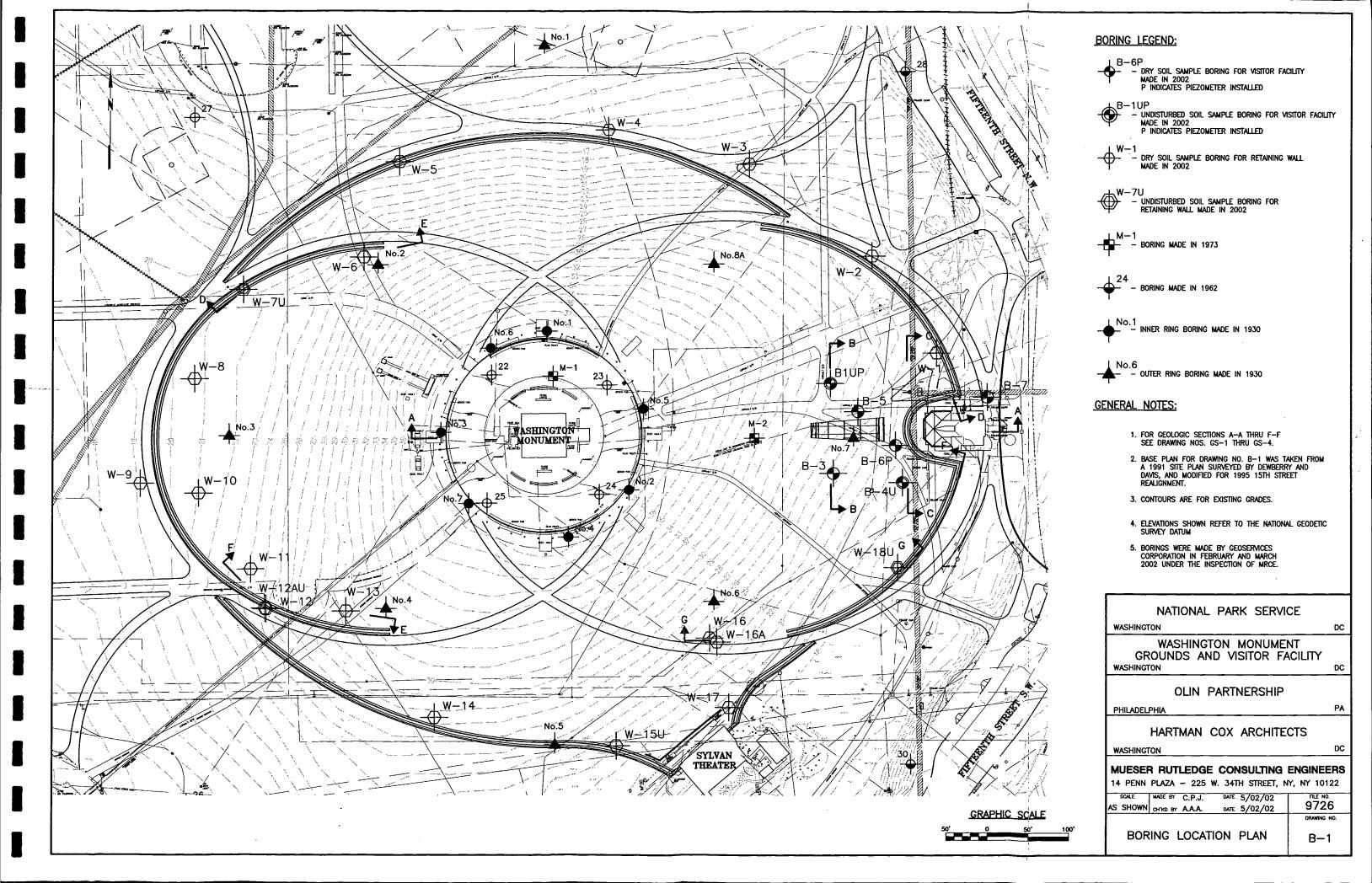
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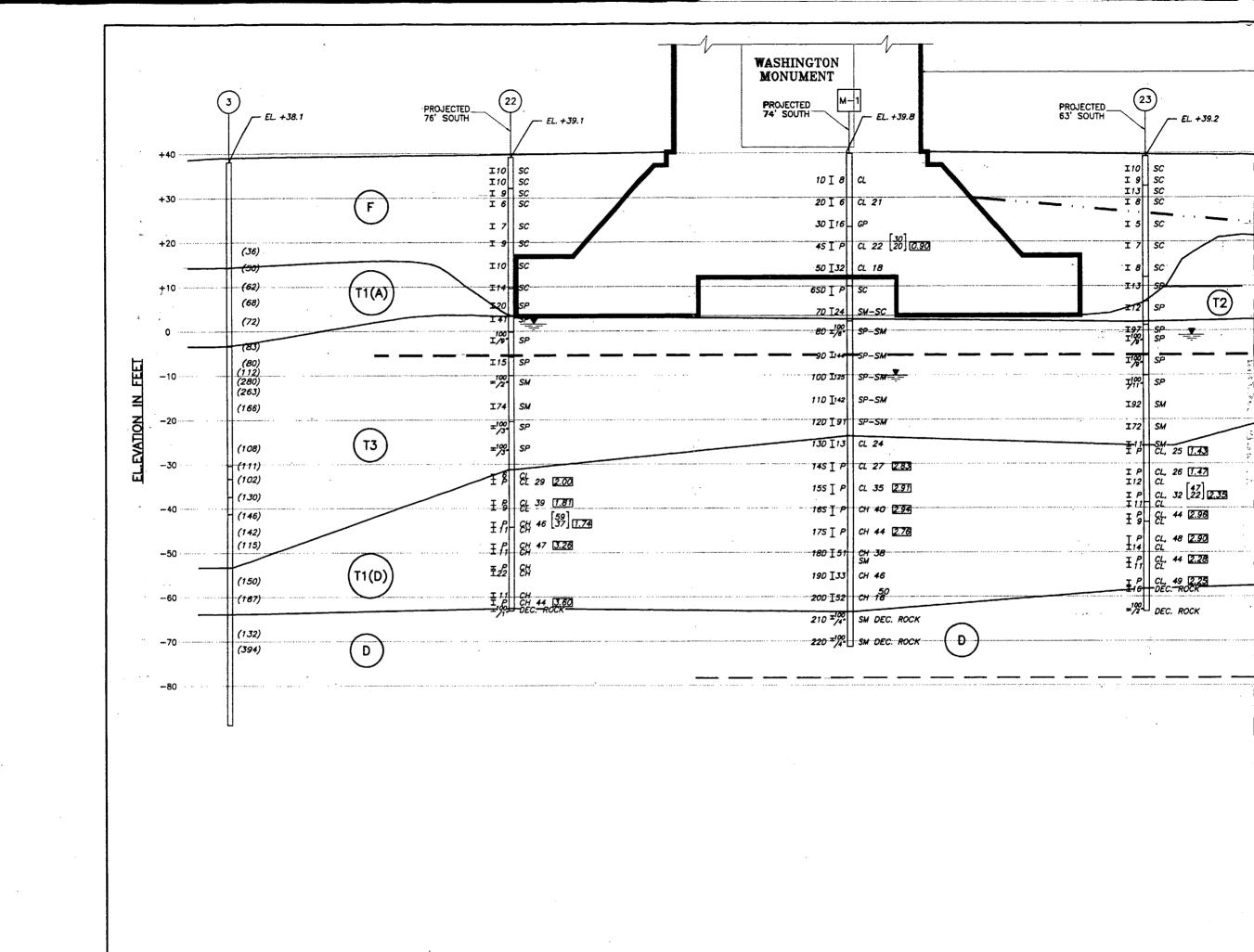
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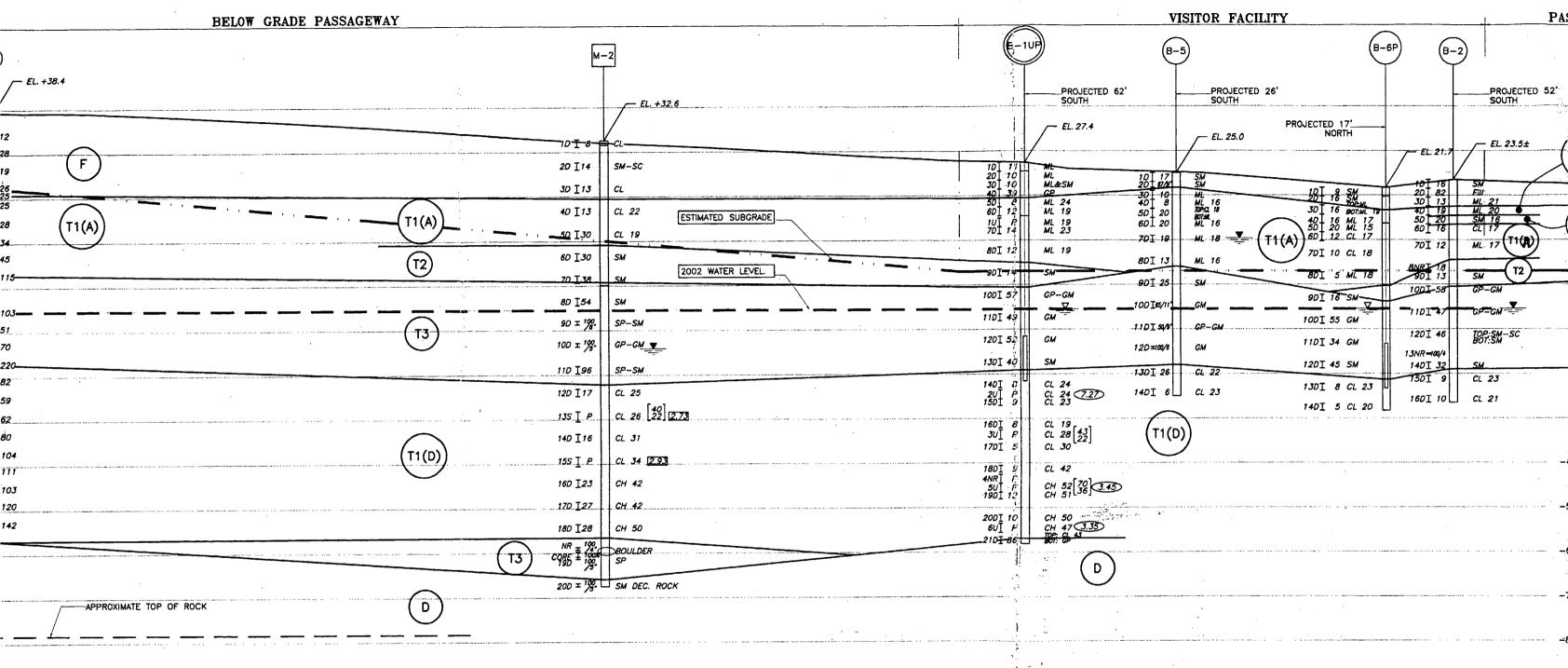
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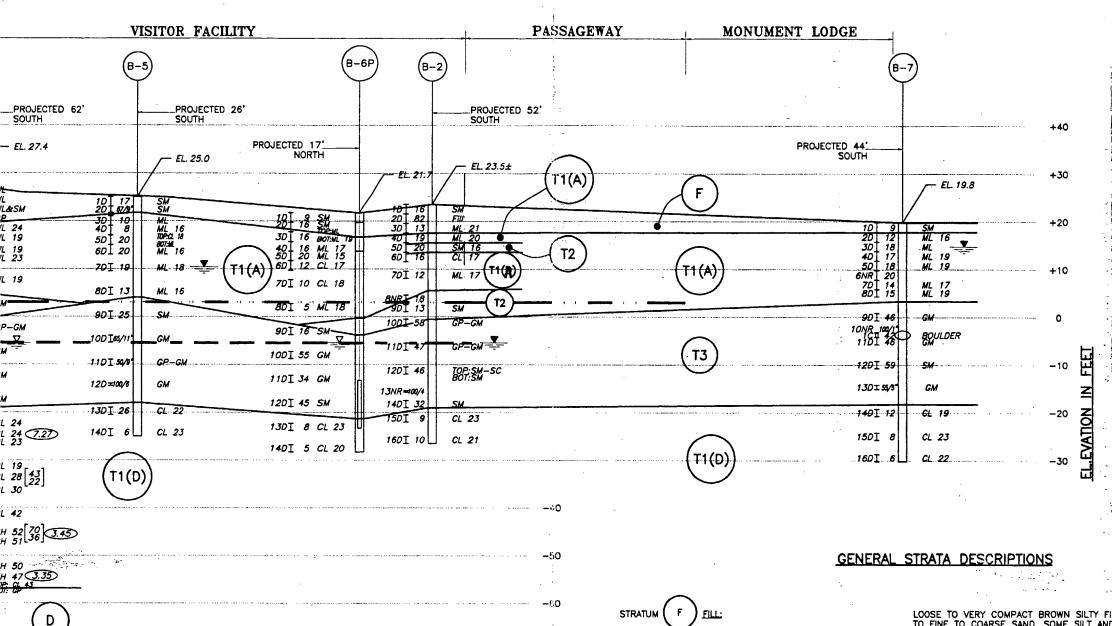
EXHIBITS







SECTION A-A



STRATUM

STRATUM

STRATUM

STRATUM

STRATUM

T2

T3

(TERRACE DEPOSITS) SANDY SILT:

(TERRACE DEPOSITS) SILTY SAND:

(TERRACE DEPOSITS) SAND AND GRAVEL:

(TERRACE DEPOSITS) PLASTIC CLAY:

NOTES:

- 1. FOR GENERAL NOTES AND BORING LOCATION PLAN SEE DRAWING NO. B-1.
- 2. STRATIFICATIONS SHOWN ON GEOLOGIC SECTIONS ARE INFERRED BETWEEN AND BEYOND THE ILLUSTRATED BORINGS AND MAY OR MAY NOT ACCURATELY REPRESENT TRUE SUBSURFACE CONDITIONS.
- 3. DRILLING MUD LEVEL:S MEASURED IN BORINGS MAY OR MAY NOT REPRESENT ACTUAL GROUND WATER LEVEL:S.
- 4. SOIL CLASSIFICATIONS WERE MADE BY MRCE AND MAY NOT AGREE WITH THE DRILLER'S CLASSIFICATIONS.
- SEE DRAWING GS-R FOR BORING LEGEND AND SUMMARY OF UNIFIED SOIL CLASSIFICATION SYSTEM.

LOOSE TO VERY COMPACT BROWN SILTY FINE TO MEDIUM SAND GRADING TO FINE TO COARSE SAND, SOME SILT AND FINE SANDY SILT, TRACE TO SOME GRAVEL, TRACE BRICK, CONCRETE, CLAY, VEGETATION, SHELLS.

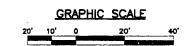
LOOSE TO MEDIUM COMPACT BROWN FINE SANDY SILT, TRACE CLAY, CLAY POCKETS, GRAVEL., LIGNITE OR STIFF BROWN CLAYEY SILT TO SILTY CLAY TRACE TO SOME FINE SAND, TRACE GRAVEL, LENSED WITH SILTY FINE SAND, AND FINE SANDY CLAY.

MEDIUM COMPACT BROWN SILTY FINE TO MEDIUM SAND, GRADING TO FINE TO MEDIUM SAND, SOME SILT, TRACE CLAY, GRAVEL.

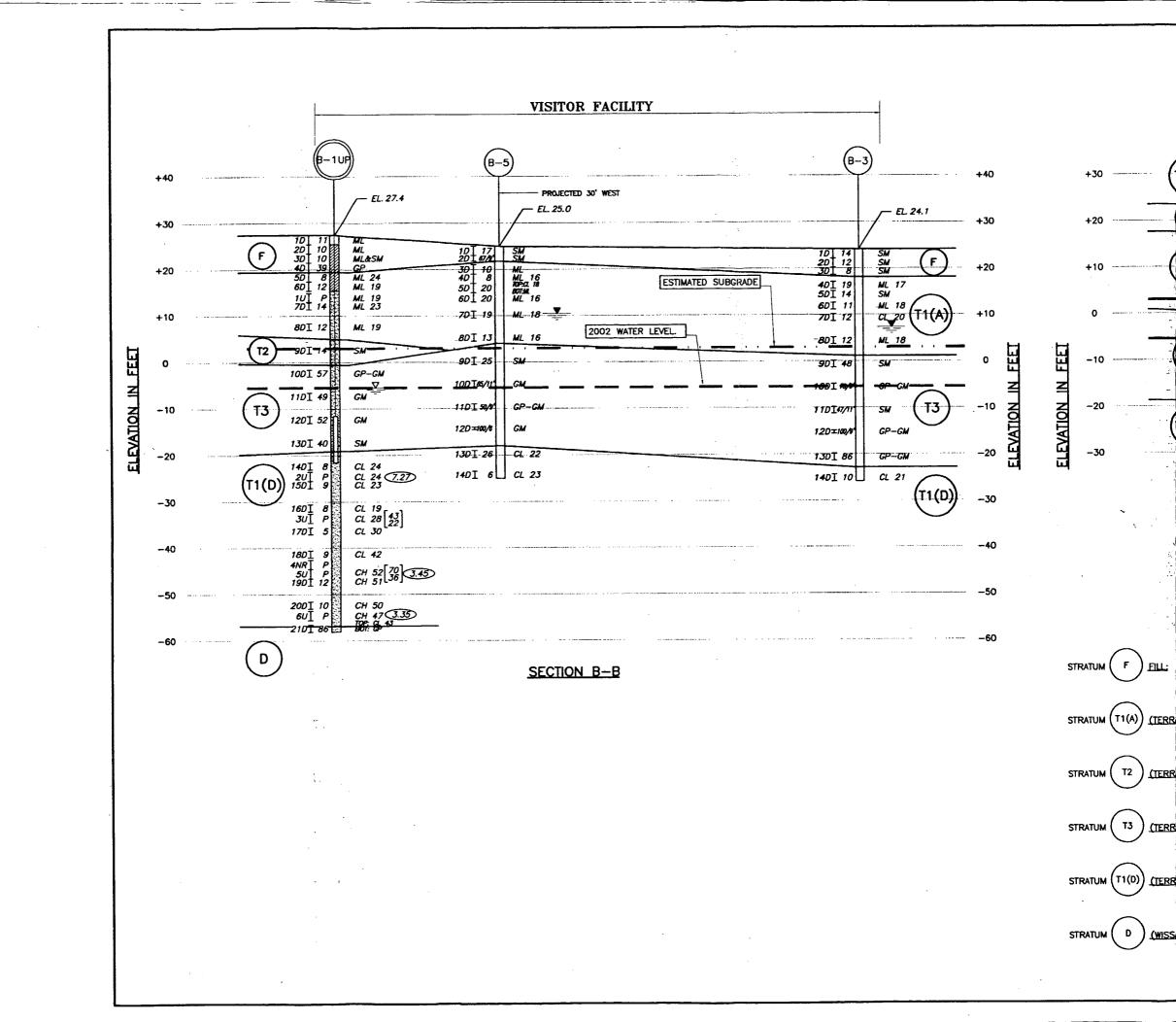
COMPACT TO VERY COMPACT BROWN FINE TO COARSE SANDY GRAVEL, TRACE TO SOME SILT, GRADING TO GRAVELLY FINE TO COARSE SAND, SOME SILT, WITH OCCASIONAL BOULDERS AND COBBLES

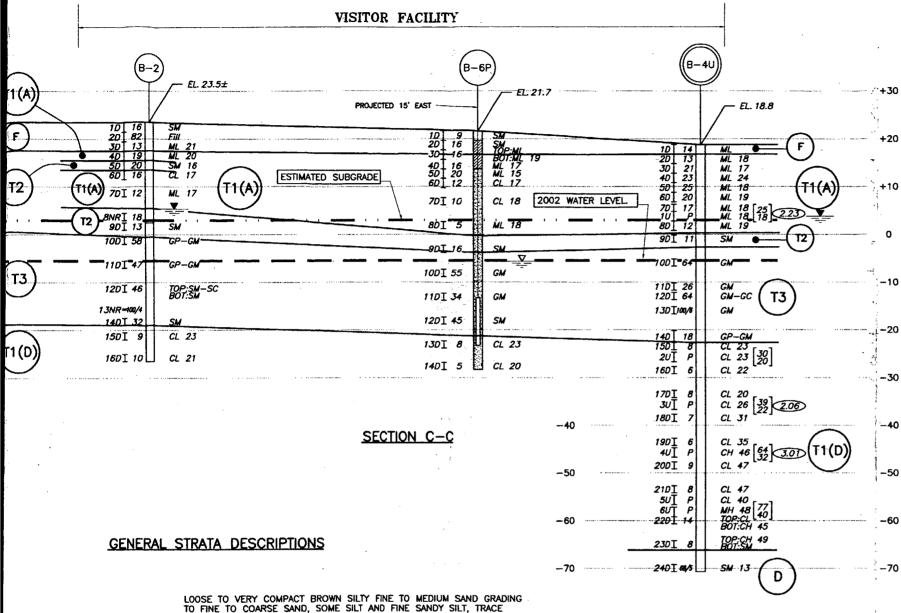
SOFT TO STIFF GRAY PLASTIC CLAY TO SILTY CLAY, TRACE TO SOME FINE SAND, TRACE FINE SAND LAYERS AND POCKETS, GRAVEL.

(WISSAHICKON SCHIST) DECOMPOSED ROCK: VERY COMPACT GRAY MICACEOUS FINE TO MEDIUM SAND, SOME SILT, TRACE TO SOME ROCK FRAGMENTS



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HARTMAN COX ARCHITE	CTS						
WASHINGTON	DC						
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GEOLOGIC SECTION A-A	GS-1						





LOOSE TO VERY COMPACT BROWN SILTY FINE TO MEDIUM SAND GRADING TO FINE TO COARSE SAND, SOME SILT AND FINE SANDY SILT, TRACE TO SOME GRAVEL, TRACE BRICK, CONCRETE, CLAY, VEGETATION, SHELLS.

ACE DEPOSITS) SANDY SILT:

ACE DEPOSITS) SILTY SAND:

LOOSE TO MEDIUM COMPACT BROWN FINE SANDY SILT , TRACE CLAY, CLAY POCKETS, GRAVEL, LIGNITE OR STIFF BROWN CLAYEY SILT TO SILTY CLAY TRACE TO SOME FINE SAND, TRACE GRAVEL, LENSED WITH SILTY FINE SAND,

MEDIUM COMPACT BROWN SILTY FINE TO MEDIUM SAND, GRADING TO FINE TO MEDIUM SAND, SOME SILT, TRACE CLAY, GRAVEL

ACE DEPOSITS) SAND AND GRAVEL:

COMPACT TO VERY COMPACT BROWN FINE TO COARSE SANDY GRAVEL, TRACE TO SOME SILT, GRADING TO GRAVELLY FINE TO COARSE SAND, SOME SILT, WITH OCCASIONAL BOULDERS AND COBBLES;

ACE DEPOSITS) PLASTIC CLAY:

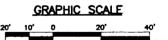
SOFT TO STIFF GRAY PLASTIC CLAY TO SILTY CLAY, TRACE TO SOME FINE SAND, TRACE FINE SAND LAYERS AND POCKETS, GRAVEL

AHICKON SCHIST) DECOMPOSED ROCK: VERY COMPACT GRAY MICACEOUS FINE TO MEDIUM SAND, SOME SILT, TRACE TO SOME ROCK FRAGMENTS

NOTES:

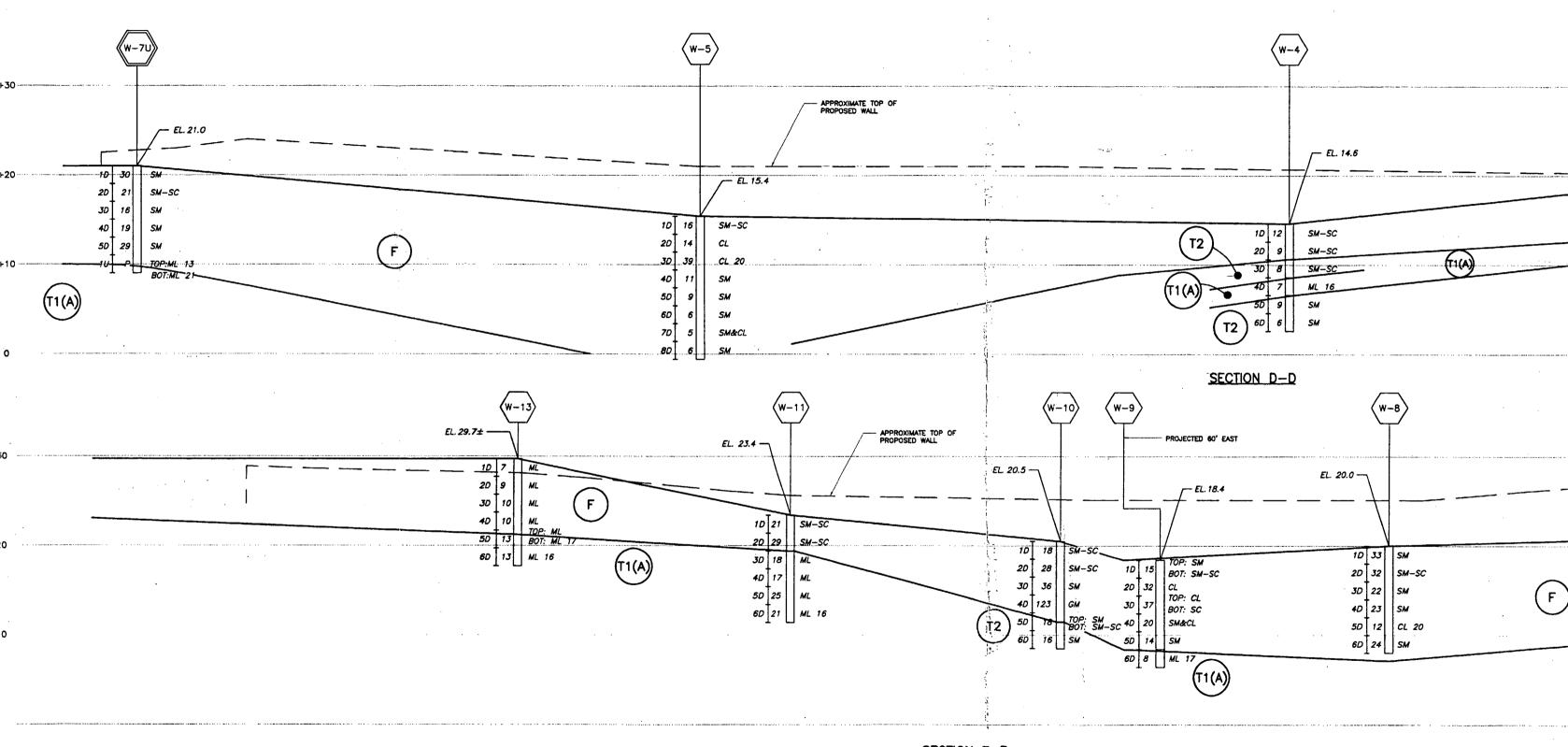
ELEVATION

- 1. FOR GENERAL NOTES AND BORING LOCATION PLAN SEE DRAWING NO. B-1.
- 2. STRATIFICATIONS SHOWN ON GEOLOGIC SECTIONS ARE INFERRED BETWEEN AND BEYOND THE ILLUSTRATED BORINGS AND MAY OR MAY NOT ACCURATELY REPRESENT TRUE SUBSURFACE CONDITIONS.
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- 4. SOIL CLASSIFICATIONS WERE MADE BY MRCE AND MAY NOT AGREE WITH THE DRILLER'S CLASSIFICATIONS.
- 5. SEE DRAWING GS-R FOR BORING LEGEND AND SUMMARY OF UNIFIED SOIL CLASSIFICATION SYSTEM.

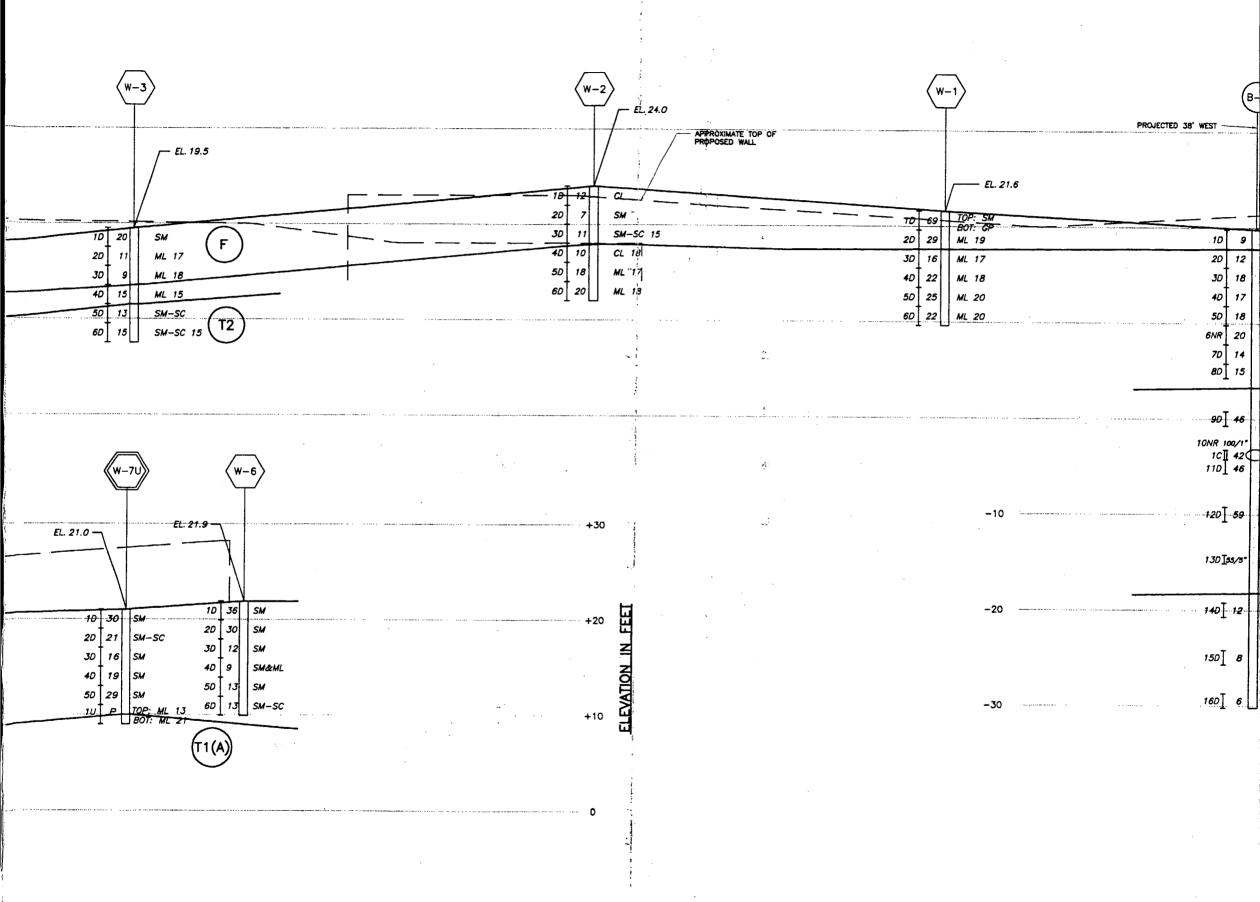


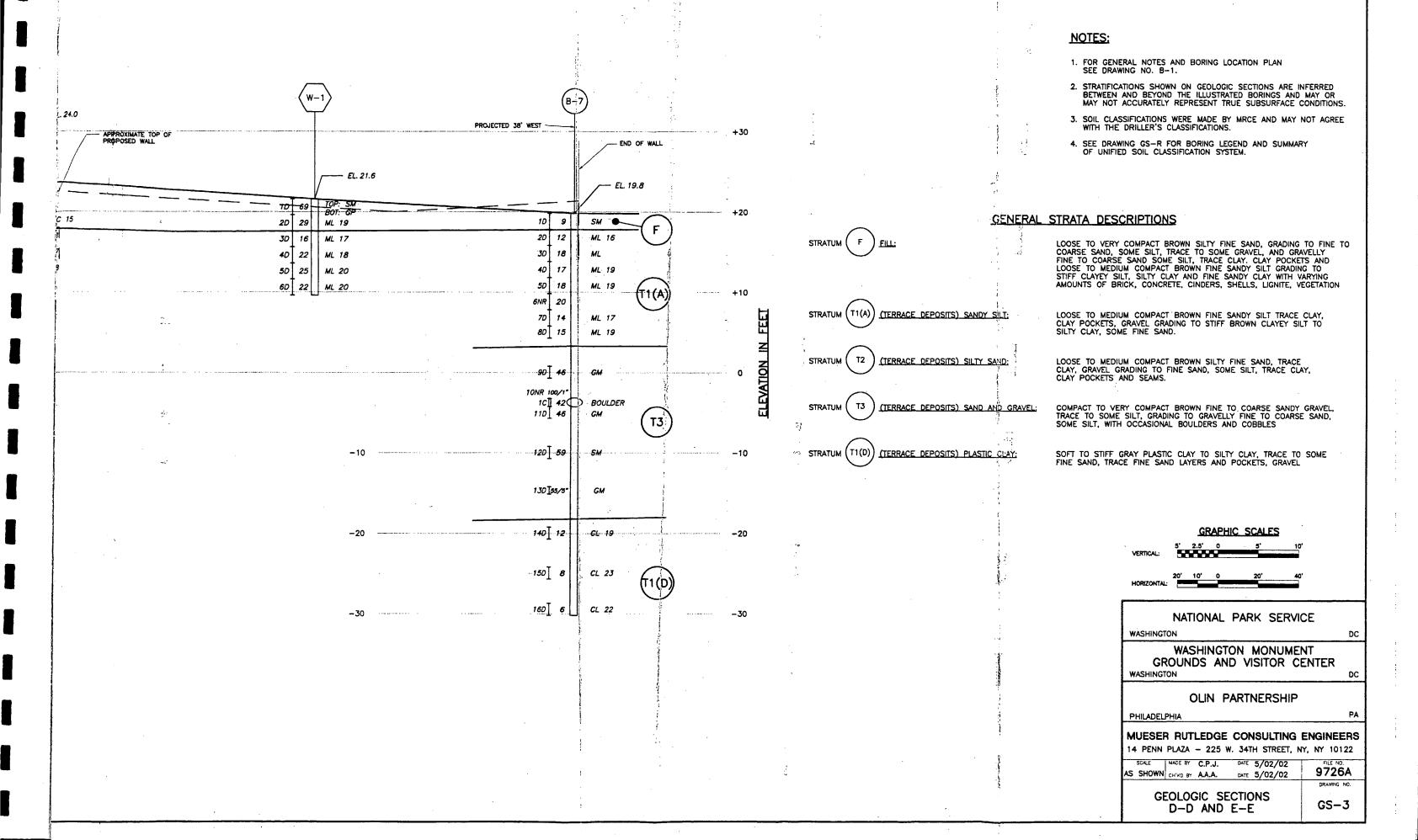
NATIONAL PARK SERVICE WASHINGTON DC WASHINGTON MONUMENT GROUNDS AND VISITOR FACILITY WASHINGTON DC HARTMAN COX ARCHITECTS DC WASHINGTON MUESER RUTLEDGE CONSULTING ENGINEERS 14 PENN PLAZA - 225 W. 34TH STREET, NY, NY 10122

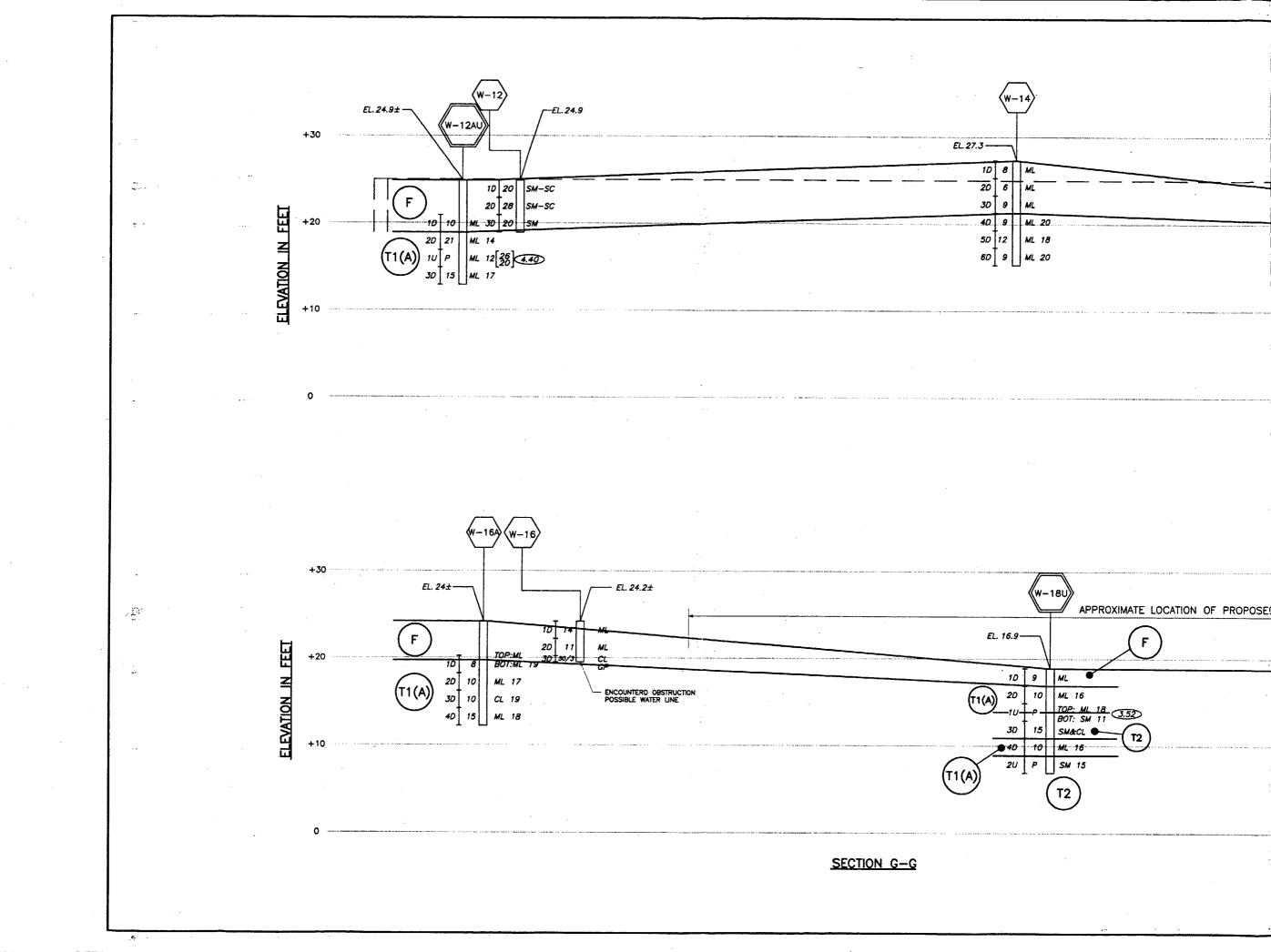
MADE BY C.P.J. DATE 5/02/02 9726B AS SHOWN CH'KD BY A.A.A. DATE 5/02/02 **GEOLOGIC SECTIONS** GS-2 B-B AND C-C

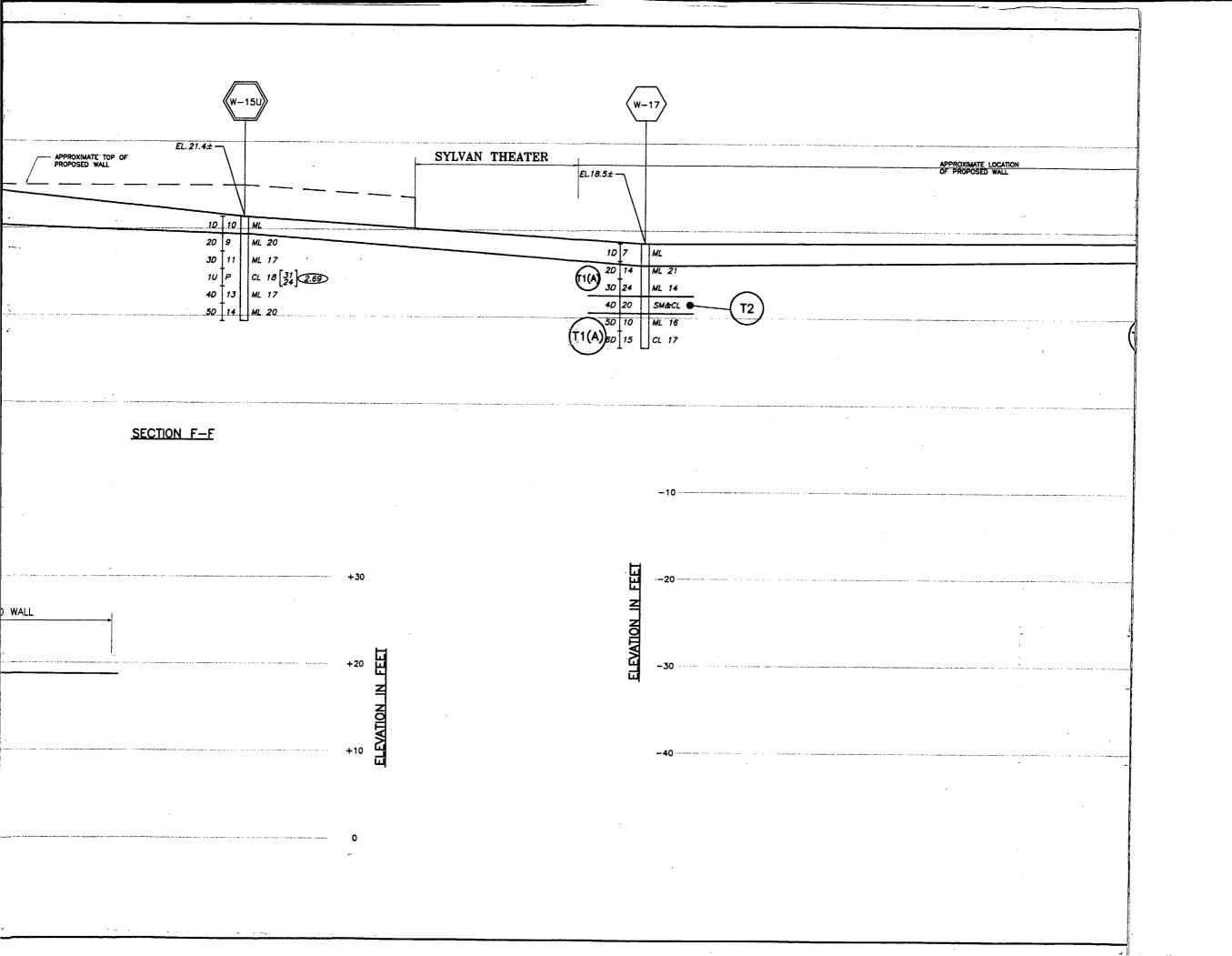


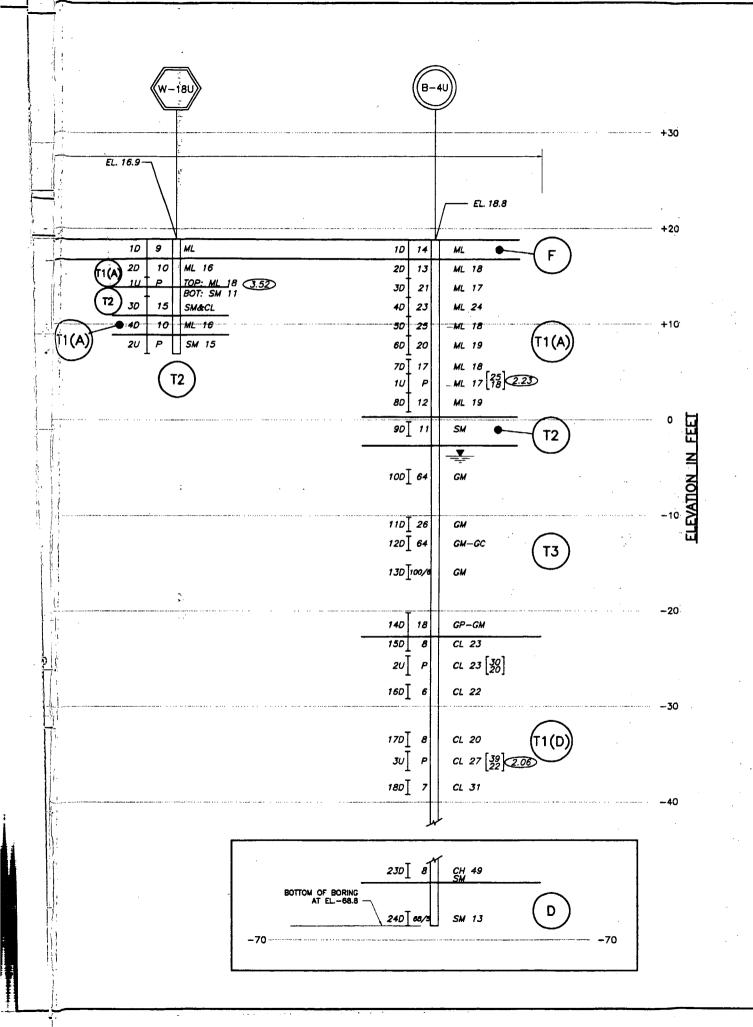
SECTION E-E











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- 1. FOR GENERAL NOTES AND BORING LOCATION PLAN SEE DRAWING NO. B-1.
- STRATIFICATIONS SHOWN ON GEOLOGIC SECTIONS ARE INFERRED BETWEEN AND BEYOND THE ILLUSTRATED BORINGS AND MAY OR MAY NOT ACCURATELY REPRESENT TRUE SUBSURFACE CONDITIONS.
- 3. SOIL CLASSIFICATIONS WERE MADE BY MRCE AND MAY NOT AGREE WITH THE DRILLER'S CLASSIFICATIONS.
- 4. SEE DRAWING GS-R FOR BORING LÉGEND AND SUMMARY OF UNIFIED SOIL CLASSIFICATION SYSTEM.

GENERAL STRATA DESCRIPTIONS

TRATUM (F) FILL:

LOOSE TO VERY COMPACT BROWN SILTY FINE SAND, GRADING TO FINE TO COARSE SAND, SOME SILT WITH FINE SANDY SILT, TRACE TO SOME GRAVEL, AND GRAVELLY FINE TO COARSE SAND SOME SILT, TRACE CLAY. CLAY POCKETS AND LOOSE TO MEDIUM COMPACT BROWN FINE SANDY SILT GRADING TO STIFF CLAYEY SILT, SILTY CLAY AND FINE SANDY CLAY WITH VARYING AMOUNTS OF BRICK, CONCRETE, CINDERS, SHELLS, LIGNITE, VEGETATION.

STRATUM (T1(A)) (TERRACE DEPOSITS) SANDY SILT:

LOOSE TO MEDIUM COMPACT BROWN FINE SANDY SILT TRACE CLAY, CLAY POCKETS, GRAVEL GRADING TO STIFF BROWN CLAYEY SILT TO SILTY CLAY, SOME FINE SAND.

STRATUM (T2) (TERRACE DEPOSITS) SILTY SAND:

LOOSE TO MEDIUM COMPACT BROWN SILTY FINE SAND, TRACE CLAY, GRAVEL GRADING TO FINE SAND, SOME SILT, TRACE CLAY, CLAY POCKETS AND SEAMS.

STRATUM (T3) (TERRACE DEPOSITS) SAND AND GRAVEL:

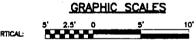
COMPACT TO VERY COMPACT BROWN FINE TO COARSE SANDY GRAVEL, TRACE TO SOME SILT, GRADING TO GRAVELLY FINE TO COARSE SAND, SOME SILT, WITH OCCASIONAL BOULDERS AND COBBLES.

STRATUM (T1(D)) (TERRACE DEPOSITS) PLASTIC CLAY:

SOFT TO STIFF GRAY PLASTIC CLAY TO SILTY CLAY, TRACE TO SOME FINE SAND, TRACE FINE SAND LAYERS AND POCKETS, GRAVEL.

STRATUM D (WISSAHICKON SCHIST) DECOMPOSED ROCK:

VERY COMPACT GRAY MICACEOUS FINE TO MEDIUM SAND, SOME SILT, TRACE TO SOME ROCK FRAGMENTS.



HORIZONTAL: 20' 10' 0 20' 40'

NATIONAL PARK SERVICE

WASHINGTON

WASHINGTON MONUMENT GROUNDS AND VISITOR FACILITY WASHINGTON

OLIN PARTNERSHIP

DEI DUIA

MUESER RUTLEDGE CONSULTING ENGINEERS
14 PENN PLAZA - 225 W. 34TH STREET, NY, NY 10122

SCALE MADE BY C.P.J. DATE 5/02/02 | FILE NO.

AS SHOWN OUTD BY AAA. DATE 5/02/02 | 9726A

AS SHOWN CHIED BY A.A.A. DATE 5/02/02

GEOLOGIC SECTIONS
F-F AND G-G

GS-4

	MAJOR DIVISIONS		GROUP SYMBOLS	TYPICAL NAMES	(EXCLUDING	ENTIFICATION PROCEDUR PARTICLES LARGER THA ACTIONS ON ESTIMATED	AN 3 IN.		
1	2_		3	4		丁二	HYDROL		
	₩ 4	AN FELS NO FINES)	GW	WELL GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES.		AIN SIZES AND SUBSTAN TERMEDIATE PARTICLE S		100	#
COARSE—GRAINED SOILS MATERAL IS LARGER THAN NO. 200 SIEVE SIZE TO THE NAKED EYE MORE THAN HALF OF COARSE FRACTION IS LARGER THAN NO. 4 SIEVE SIZE. ATION, THE 1/4 —IN. SIZE MAY BE USED AS IT TO THE NO. 4 SIEVE SIZE) GRAVELS WITH GRAVELS WITH GRAVELS WITH GRAVELS NO FINES) OF FINES OF FINES		CLE GRAN (LITTLE OR	GP	POORLY GRADED GRAVELS, GRAVEL-SAND MIXTURES, LITTLE OR NO FINES.		NE SIZE OR A RANGE O TERMEDIATE SIZES MISSI		80 70	
		S WITH ES CABLE UNT NES)	GM .	SILTY GRAVELS, GRAVEL—SAND—SILT—MIXTURES.	SILTY GRAVELS, GRAVEL—SAND—SILT—MIXTURES. NONPLASTIC FINES OR FINES WITH LOW PLASTICITY (FOR IDENTIFICATION PROCEDURES SEE ML BELOW				Ri C ₁
			œ	CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES. PLASTIC FINES (FOR IDENTIFICATION PROCEDURES SEE CL BELOW)				PER CENT	c,
			SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES.		AIN SIZES AND SUBSTAN TERMEDIATE PARTICLE S		20	#1
NORE THAN HALF OF WATERAL SMALLEST PARTICLE VISIBLE TO THE	SANDS MORE THAN HALF OF COARSE FRACTION IS SWALLER THAN NO. 4 SIEVE SIZE. (FOR VISUAL CLASSIFICATION, THE EQUINALENT TO THE N	CLEAN SANDS (LITTLE OR NO FINES)	SP	POORLY GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES.	PREDOMINANTLY ON SOME INT			1 .0	
	SANDS RE THAN HALF (RON IS SWALLER SIEVE SIZ (FOR VISUAL	SANDS WITH FINES (APPRECABLE AMOUNT OF FINES)	SM	SILTY SANDS, SAND-SILT-MIXTURES.	NONPLASTIC FINE (FOR IDENTIFICAT	S OR FINES WITH LOW ION PROCEDURES SEE	PLASTICITY ML BELOW)	Laves	HCATIO
ABOUT THE S	FRAC	SANDS WITH FINES (APPRECIABL AMOUNT OF FINES)	SC	CLAYEY SANDS, SAND-CLAY MIXTURES.	(FOR IDENTIFICAT	PLASTIC FINES TION PROCEDURES SEE	CL BELOW)		
SIZE SIZE IS						RFICATION PROCEDURES MALLER THAN NO. 40 S		6	60
8 8	:	!			DRY STRENGTH (CRUSHING CHARACTERISTICS)	DILATANCY (REACTION TO SHAKING)	TOUGHNESS (CONSISTENCY NEAR PL)] •	50
THAN -	St 55 8		ML	INORGANIC SILTS, SANDY SILTS, ROCK FLOUR, OR CLAYEY SILTS WITH SLIGHT PLASTICITY.	none to slight.	QUICK TO SLOW	NONE		
IS SMALLER THAN NO. 7	SILTS AND CLAYS LIQUID LIMIT IS LESS THAN 50		CL	INORCANIC CLAYS, OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS.	MEDIUM TO HIGH	NONE TO VERY SLOW	MEDIUM	T SDEX 4	
WATERIAL IS	SILIS	ļ	OL.	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY.	SLIGHT TO MEDIUM	SLOW	SUGHT	PLASTICITY 2	
₽	LAYS I SO IN 50		мн	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SANDY OR SILTY SOILS, ELASTIC SILTS.	SLICHT TO MEDIUM	SLOW TO NONE	SLICHT TO MEDIUM		
Than Half	SILTS AND CLAYS LÍQUID LIMIT IS GREATER THAN 50	!	СН	INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS.	HIGH TO VERY HIGH	NONE	нсн	2	20
MORE	SILTS LIQU OREA		OH ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS.		MEDIUM TO HIGH NONE TO VERY SLIGHT TO SLOW MEDIUM]],	10
	HIGHLY ORGANIC SOIL	ıLS	Pt	PEAT AND OTHER HIGHLY ORGANIC SOILS.		D BY COLOR, ODOR, SP JENTLY BY FIBROUS TEX			7 4

TERMINOLOGY USED IN MRCE SOIL DESCRIPTIONS

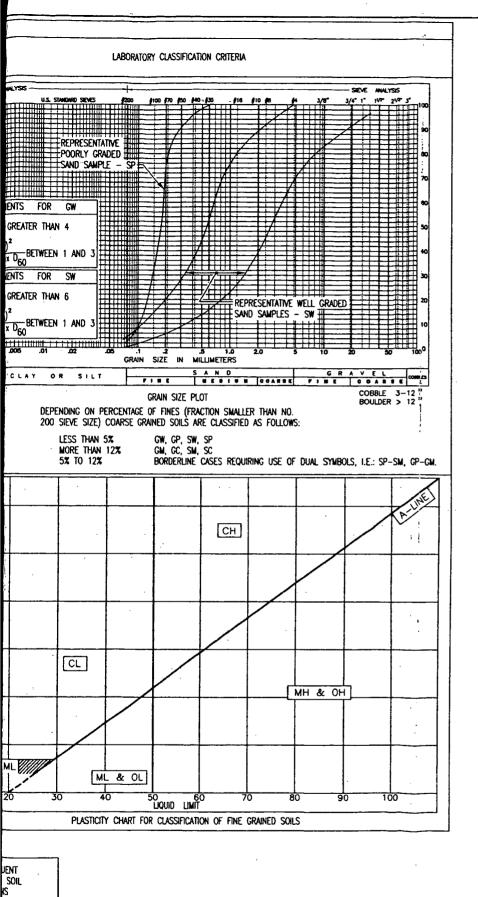
DESCRIPTION OF CONSTIT PERCENTAGES AS USED IN SAMPLE CLASSIFICATIO

> "AND" (EG. SAND

1% to 12% - "Trace";
13% to 30% - "Some"
31% to 49% - Adjective
Soil gro
(EG. Sant

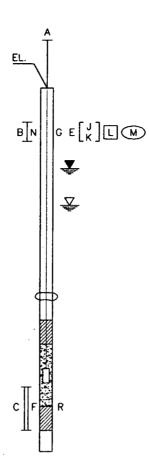
EQUAL AMOUNT -

	DEGREE OF COMPACTION	FOR NON-PLASTIC SOIL		Consistency of Clay and Clayey Silt +							
	DEGREE OF COMPACTION	Blows* Per foot	CONSISTENCY	UNCONFINED COMPRESSIVE STRENGTH (TSF)	IDENTIFICATION CHARACTERISTICS						
Γ	LOOSE	0 то 10	SOFT	LESS THAN 0.5	EASILY REMOLDED WITH SLIGHT FINGER PRESSURE						
	MEDIUM COMPACT	11 то 29	MEDIUM	0.5 то 1.0	REQUIRES SUBSTANTIAL PRESSURE FOR REMOLDING	ĺ					
	COMPACT	30 то 50	STIFF	1.0 то 4.0	DIFFICULT TO REMOLD WITH FINGERS						
	VERY COMPACT	greater than 50	HARD	GREATER THAN 4.0	CANNOT BE REMOLDED WITH FINGERS						
	 STANDARD PENETRATION RESIST HAMMER FREE FALLING 30 INC 0.D. SPLIT—SPOON SAMPLER. 	ANCE USING 140 LB. HES TO DRIVE A 2 INCH		ARE DESCRIBED USING DEGREE OF COM R NON-PLASTIC SOIL.	IPACTION .						



FORM OF

AND GRAVEL)



BORING LEGEND

- A NUMBER, TYPE AND LOCATION OF BORING
- EL. GROUND SURFACE ELEVATION AT BORING
- B NUMBER AND TYPE OF SAMPLE
 - D DRY SAMPLE TAKEN WITH 2 INCH O.D. SPLIT SPOON
 - U UNDISTURBED SAMPLE TAKEN WITH 3 INCH O.D. FIXED PISTON TYPE SAMPLER
 - UD UNDISTURBED SAMPLE EXTRUDED IN FIELD AND PLACED IN JAR DUE TO POOR RECOVERY OR DISTURBANCE
 - S THIN TUBE SAMPLE TAKEN WITH SHELBY TUBE SAMPLER
 - W WASH SAMPLE
 - -NR NO RECOVERY
- LENGTH OF SAMPLE ATTEMPT
 - STANDARD PENETRATION RESISTANCE.
 NUMBER OF BLOWS FROM 140 LB. HAMMER FREE FALLING 30 INCHES REQUIRED TO DRIVE 2 INCH 0.D. SPLIT SPOON SAMPLER ONE FOOT AFTER INITIAL PENETRATION OF 6 INCHES, UNLESS A SPECIFIC PENETRATION IS INDICATED.
 - P PRESSED OR PUSH SAMPLE
 - NH SAMPLE TAKEN UNDER WEIGHT OF HAMMER AND RODS
 - WR SAMPLE TAKEN UNDER WEIGHT OF RODS
- AVERAGE NATURAL WATER CONTENT OF SAMPLE, IN PERCENT OF DRY WEIGHT
- G UNIFIED SOIL CLASSIFICATON GROUP SYMBOL OF SAMPLE
- [J] = ATTERBERG LIQUID LIMIT VALUE K] = ATTERBERG PLASTIC LIMIT VALUE
- COMPRESSIVE STRENGTH IN TSF DETERMINED FROM UNCONFINED COMPRESSION TEST
- COMPRESSIVE STRENGTH IN 1SF DETERMINED FROM UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TEST
- GROUNDWATER LEVEL OBSERVED IN BORING

 * MUD LEVEL
- 🛂 Groundwater level observed in Piezometer
- C ROCK CORE NUMBER
- LENGTH OF CORE RUN
- LENGTH OF CORE RECOVERED EXPRESSED AS A PERCENT OF THE LENGTH OF CORE RUN
- R ROCK QUALITY DESIGNATION—THE SUM OF THE LENGTHS OF PIECES OF RECOVERED CORE WHICH ARE EQUAL TO OR CREATER THAN FOUR INCHES IN LENGTH, EXPRESSED AS A PERCENTAGE OF THE TOTAL LENGTH OF CORE RUN. LENGTHS ARE MEASURED BETWEEN IN—SITU SEPARATIONS AND MECHANICAL BREAKS RESULTING FROM CORING ARE IGNORED.
- IMPERMOUS SEAL
- SAND FILTER SURROUNDING PIEZOMETER INTAKE ELEMENT
- INTAKE ELEMENT
 - COBBLE OR BOULDER

REVISED - SEPTEMBER, 2001

MUESER RUTLEDGE CONSULTING ENGINEERS
225 WEST 34TH STREET - 14 PENN PLAZA
NEW YORK, NY 10122

GEOTECHNICAL REFERENCE STANDARDS DRAWING NO.

GS-R

TABLE NO. 1

SUMMARY OF LABORATORY TEST DATA

SAI	SAMPLE IDENTIFICATION CLASSIFICATION PROPERTIES												PHYSIC	CAL PI	ROPER	TIES									
				1						CLAS	UNIFIED SOIL CLASSIFICATION SYSTEM STRENGTH			CONSOLIDATION											
BORING NUMBER	SAMPLE NUMBER	ELEVATION, FT.	STRATUM DESIGNATION	SOIL TYPE	AVERAGE NATURAL WATER CONTENT, Wn , %	LIQUID LIMП, W _L , %	PLASTICITY INDEX, Ip , %	NATURAL WATER CONTENT OF LIMIT SAMPLE, Wn , %	SPECIFIC GRAVITY OF SOLIDS, Gs	GROUP SYMBOL	% SAND (< #4 > #200 SIEVE)	% FINES (< #200 SIEVE)	TYPE OF TEST	COMPRESSIVE STRENGTH (σ ₁ -σ ₃), TSF	CONFINING PRESSURE	STRAIN AT FAILURE, %	NATURAL WATER CONTENT, Wn , %	WATER CONTENT AT END OF TEST, W _f , %	NATURAL WATER CONTENT, Wn , %	INITIAL VOID RATIO, 60	EXISTING OVERBURDEN STRESS, R, , TSF	ESTIMATED PRECONSOLIDATION STRESS, R, TSF	COMPRESSION INDEX, Cc	SWELLING INDEX, C _s	VOID RATIO AT START OF SWELL, er
B-1UP	9D 2U 3U 5U 6U	2.7 -24.6 -33.6 -45.6 -54.6	T2 T1(D) T1(D) T1(D) T1(D)		24 27 49 47	43 70	21 34	27.5 45.6	2.98 2.73 2.66	SM CL CH	70	28	UU UU	7.27 3.45 3.35	2.22 2.79 3.04	15.8 4.0 4.1	23.9 46.3 47.2	23.9 46.2 46.9	26.8 46.2	0.736 1.270	3.4 3.5	7.0 8.5	0.34 0.74	0.09 0.15	0.382 0.631
B-4U	1U 2U 3U 4U 6U	3.8 -25.7 -35.7 -45.7 -58.2	T1(A) T1(D) T1(D) T1(D) T1(D)		18 23 26 46 48	25 30 39 64 77	7 10 17 32 37	19.5 23.3 26.0 52.0 47.1	2.70 2.72	CL-ML CL CH MH			บบ บบ บบ	2.23 2.06 3.01	0.78 2.09 2.35	7.2 6.3 4.5	18.7 25.3 43.8	18.9 25.3 44.7	47.3	1.360	3.4	9.5	0.75	0.16	0.68
W-7U	1U	10.0	T1(A)		17								UU	2.15	0.57	10.8	17.6	17.5							
N-12AU	1U	15.9	T1(A)		12	25.9	6	12.3		CL-ML			υU	4.40	0.47	2.4	12.9	13.4							
W -15U	10	14.4	T1(A)		18	31	9	19.3		CL			UU	2.65	0.37	4.0	18.5	18.8							
W -18U	1U 2U	11.9 5.9	T1(A) T2		18 15					SM	56	44	UU	3.52	0.26	6.3	19.3	19.4							
				<u>e</u> ,	TRATA D	DESIGNA	ATIONS											NO.	TES						

T1(A) - SANDY SILT

T2 - SILTY SAND

T1(D) - PLASTIC CLAY

- All tests summarized were performed in the laboratory of Mueser Rutledge Consulting Engineers.
- The sample elevation is the average of the sampling interval.
- 3. Ground surface elevations at borings are:

BORING NO.	GROUND SURFACE ELEVATION
B-1UP	27.4
B-4UP	18.8
W-7U	21.0
W-12AU	24.9
W-15U	21.4
W-18U	16.9

- 4. "Average natural water content" is a weighted average of all material tested.
- 5. Strength tests performed were:
 - UU Unconsolidated Undrained Triaxial Compression
- Strength tests were performed on a specimen 2.8 inches in diameter with a height to diameter ratio of approximately 2 at a rate of strain of approximately 1% per minute.
- Confining pressure for UU and CU compression tests is equivalent to 80 percent of estimated vertical effective overburden stress, unless otherwise noted.
- 8. Compression Index, Cc = slope of the virgin compression portion of the e-log p curve.
- 9. Recompression Index, Cs = slope of the rebound portion of the e-log p curve.
- Most probable preconsolidation stress, Pc is determined by the Casagrande method of construction.
- 11. Minimum preconsolidation stress, Pmin, is the pressure ordinate at the intersection of the extension of the virgin compression portion of the e-log P curve with the initial void ratio.

MUESER RUTLEDGE CONSULTING ENGINEERS 225 WEST 34TH STREET, NEW YORK, N.Y. 10122

WASHINGTON MONUMENT

WASHINGTON DC

FILE NO. 9726A&B

SHEET 1 OF 1

TABLE NO. 1

APPENDIX A

MUESER RUTLEDGE CONSULTING ENGINEERS BORING LOG

PROJECT:	W
LOCATION	

WASHINGTON MONUMENT VISITOR CENTER

WASHINGTON, D.C.

BORING NO. B-1UP

SHEET 1 OF 4

FILE NO. 9726B

SURFACE ELEV. 27.4

RES. ENGR. MICHAEL WECKLER

						RES	. ENGR.	MICHAEL WECKLER
DAILY		SAMPL	E				CASING	
PROGRESS	NO.	DEPTH	BLOWS/6"	SAMPLE DESCRIPTION	STRATA	DEPTH	BLOWS	REMARKS
09:20	1D	0.0	4-7	Brown fine to coarse sandy silt, trace			18	
03-05-02		2.0	4-5	vegetation (Fill) (ML)			17	
Tuesday	2D	2.0	4-4	Red brown fine sandy silt, trace clay, brick			21	
Clear		4.0	6-21	(Fill) (ML)	F	-	70	*Drilled ahead of
30°F	3D	4.0	4-5	Brown fine sandy silt & silty fine sand, trace	1	5	- , -	casing then drove
30 1	30	6.0	5-9	gravel (Fill) (ML&SM)				casing their drove
	4D	6.0	10-10	White quartz gravel, some fine to coarse			10*	casing.
1	40	8.0	29-8	sand, trace silt (Fill) (GP)		8	83*	
	5D	8.0	4-4	Stiff brown clayey silt, some fine sand (ML)		-		WC=24
	30	10.0	4-6	Sum brown dayby sint, some time sand (WL)		10	106*	VVO=24
•	6D		3-5	Brown fine sandy silt, trace clay (ML)		1.0		nn_1
	_ טס	10.0	7-9	blown line sailty siit, trace clay (WL)				pp=1.5 WC=19
ì	411	12.0	í	Do SD (ML)			18*	4 E WC 10
	1U	12.5	PUSH=23"	Do 6D (ML)				pp>4.5, WC=19
		14.4	REC=22"	Drawn fine conductit trace clay (A41)	~~ (^)	4 5	19*	WO 00
	7D	14.5	4-5	Brown fine sandy silt, trace clay (ML)	T1(A)	15		WC=23
	_	16.0	9			\vdash	40*	
•		· .					25*	
		A.				<u> </u>	26*	
							27*	
, j	8D	19.0	4-6	Do 7D (ML)	ĺ	20		WC=19
		20.5	6				34*	
n							35*	
					*	22.5	30*	
•							33*	
	9D	24.0	3-6	Brown silty fine to medium sand, trace clay,	1	25	34*	
		25.5	8	clay pockets, gravel (SM)	T2		49*	
J				·			47*	
						28	35*	
							38*	
	10D	29.0	21-24	Brown fine to coarse sandy gravel, trace silt		30	40*	
		30.5	33	(GP-GM)			42*	
							82*	
							175*	
ľ							230*	
	11D	34.0	30-19	Red brown fine to coarse sandy gravel,		35	67*	
14:30		35.5	30	some silt (GM)			95*	
15:30					T3		62*	
03-06-02							64*	
Wednesday							33*	Hard drilling, from
Clear	12D	39.0	100/6"-22	Do 11D (GM)		40		39' - 44'.
50°F		40.5	30				104*	
							135*	
							210*	
ין ו							200*	
] [13D	44.0	16-20	Brown gravelly fine to coarse sand, some		45		Hard drilling from
		45.5	20	silt (SM)	,			44'-46.5.
		70.0	20			46.5	71*	
]						70.5	70*	
							70*	
	145	40.0	0.4	Madium gray cilty clay trace grayel fine to	T1/D)	FO		nn_0.7 M/C_04
	14D	49.0		Medium gray silty clay, trace gravel, fine to	T1(D)	50		pp=0.7, WC=24
		50.5	4	medium sand pockets (CL)			REVERT	
<u> </u>	Ll					BOD!	NG NO.	B-1UP
						DUHIL	4G NU.	D-TUF

MUESER RUTLEDGE CONSULTING ENGINEERS BORING LOG

PROJECT: WASHINGTON MONUMENT VISITOR CENTER
LOCATION: WASHINGTON, D.C.

BORING NO. B-1UP

SHEET 2 OF 4

FILE NO. 9726B

SURFACE ELEV. 27.4

RES. ENGR. MICHAEL WECKLER CASING SAMPLE DAILY STRATA DEPTH BLOWS **REMARKS SAMPLE DESCRIPTION** PROGRESS DEPTH BLOWS/6" NO. USED Cont'd Stiff gray silty clay, trace fine sand, gravel REVERT pp=1.3, TV=0.6 2U 51.0 PUSH=24" 03-06-02 WC=24 (CL) **REC=10"** 53.0 Wednesday pp=0.5, WC=23 Medium grav silty clay (CL) 15D 53.0 1-4 Clear 55 54.5 5 50°F Stiff gray silty clay, some fine to medium pp=1.1, WC=19 16D 1-4 58.0 sand, tr fine to medium sand pockets (CL) 60 59.5 Stiff gray silty clay, some fine to medium pp=1.5, TV=0.8 PUSH=24" 3U 60.0 WC=28 REC=14" sand, trace gravel (CL) 62.0 Medium gray silty clay (CL) pp=1.0, WC=30 17D 63.0 2-2 65 64.5 3 T1(D) Stiff gray silty clay, some fine sand layers, pp=1.4, WC=42 18D 2-3 68.0 70 trace lignite seams (CL) 69.5 No recovery 4NR 70.0 PUSH=24" 72.0 REC=0" pp=2.8, TV=0.9 Stiff gray clay, trace fine sand (CH) 72.0 PUSH=24" 5U WC=52 74.0 **REC=17**" 75 Stiff gray clay, some fine sand, trace fine pp=2.2, WC=51 19D 74.0 3-5 sand seams (CH) 75.5 7 80 Stiff gray clay, trace fine sand, fine sand pp=2.1 WC=50 20D 79.0 3-4 seams (CH) 80.5 6 Do 20D, trace gravel (CH) pp=2.5, TV=1.1 PUSH=24" 6U 81.0 WC=47 **REC=23**" 83.0 21D Top pp=1.7 84.5 WC=43 Top: Stiff gray silty clay, trace fine sand, 21D 84.0 12-32 D fine sand seams, gravel (CL) 85.5 End of Boring at 54 14:30 85.5 Bot 12": Gray gneiss fragments (GP) 85.5'. pp=Pocket 90 Penetrometer Unconfined Compressive Strength in tsf. TV=Torvane Shear 95 Strength in tsf. WC=Water Content in percent of dry weight. 100 BORING NO.____ B-1UP

SHEET	3	_OF <i>4</i>
		9726B
SUBCO	DE	

PIEZOMETER NO. B-JUP

MUESER RUTLEDGE CONSULTING ENGINEERS

PIEZOMETER RECORD

ZATION WAS ZOMETER SEE SKETC	LOCATION _	<u></u>		D	ATE OF IN	NO. B-1UP STALLATION 3/102
SEE SILE 10.			PIEZO	METER TYP	E SLOTTE	D PVC
STRATA GROUND	PIEZOMETER INSTALLATION DETAILS FLUSH			dep	KE POINT th to botton depth to top	n, ft= <u>84.5</u> ft= <u>12.0</u>
SURFACE ELEV.	Mount		·	diame	length ter,in = 4生生	, ft = <u>72.5</u> = L , ft = <u>=================================</u>
FILL	100 000 000 000 000 000 000 000 000 000	8 0 8			3-7/8 NDPIPE/RI	SER , ft= <u>21.2</u>
*		6 8 10 7 2	·		ter, in = <u>1.5</u>	
TI(A)			READING TIME	DEPTH - RIM TO WATER	ELEVATION OF WATER	REMARKS
		20	37 02 8:15 317 02 14:00	30.7 33.9	-3.5 -5.7	FIRST RENDING
72.5'		I,5"ID; PUC STANDINE	3/8/02 15:15	32.97 33.02	-5.82	1-DAY READING
28'		30	3/12/02 13:30 3/13/02 14:30 3/14/02 13:30	32.89 32.89	-5.71 -5.69 -5.69	5-DAY READING READING FOLLOWING - 16 HR R
1.70			3/12/05 13:00 3/12/05 P:12	32.89 32.85	-5.69 -5.65	COORDINATED READING W/P
T3		17800	3/16/02 8:00	32.89	-5.69	
		NO. 20 PUC SCREE				
46.5	1:::				1	1 '
46.5		-49 50			1	

Sand Bentonite

Gravel Grout

MUESER RUTLEDGE CONSULTING ENGINEERS

BORING NO. B-1UP

				SHEET	4	OF _	4
PROJECT	WASHINGTON MOI	NUMENT VISITO	R CENTER_	FILE NO.	9	726B	
LOCATION	WASHINGTON, D.C)		SURFACE I	ELEV	27.4	
BORING LOCATION	SEE BORING LOCA	ATION PLAN		DATUM		NGSD	
BORING EQUIPMENT A	ND METHODS OF STAE	BILIZING BOREH	<u>OLE</u>				
	TYPE OF FEED	•					
TYPE OF BORING RIG	DURING CORING	G CAS	ING USED	XYES	F	NO	
TRUCK CME-55	MECHANICAL	DIA.,	IN 4	DEPTH, FT. I	FROM	o ·	го 49
SKID	HYDRAULIC	DIA.,	IN.	DEPTH, FT. I	FROM _		го
BARGE	OTHER	DIA.,	IN.	DEPTH, FT. I	FROM		го
OTHER							
					ſ		
TYPE AND SIZE OF:			NG MUD USED	X YES	Į	NO	
	LIT SPOON		ER OF ROTARY B	IT, IN.		3-7/8	
U-SAMPLER 3" SHELBY	' HEAD	TYPE OI	F DRILLING MUD			REVERT	
		41.055	LICED		Г	X NO	
	· · · · · · · · · · · · · · · · · · ·		RUSED	YES	L	X NO	
		I YPE AF	ND DIAMETER, IN.	-			
DRILL RODS AW		+0.4.0(1)(140	A\/EDAC	C [ALL IN]	30
			HAMMER, LBS.	140		GE FALL, IN. GE FALL, IN.	30
		SAMPLE	ER HAMMER, LBS.		AVENAC	JE FALL, IIV.	
WATER LEVEL OBSERV	ATIONS IN BOREHOLE			*MECHANICAL	_ HAMME	ER USED.	
	DEPTH OF DEPTH OF	DEPTH TO					,
DATE TIME	HOLE CASING (FEET)	WATER (FEET)	COND	DITIONS OF OB	SERVAT	ION	
		 ` ` 		ETION			
3-6-02 14:30	85.5 49	26.7	MUD LEVEL U				
3-7-02 5:30	85.5 49	33.7	OVERNIGHT	READING IN	BOHIN	J	
		i					
							
							
PIEZOMETER INSTALLE	D X YES	NO SKET	CH SHOWN ON		SH	HEET 3 of 4	
PIEZUWETER INGTALLE	<u>D</u> [X]120	INO OKEK	311 01101111 011				
STANDPIPE: TYPE	SCHEDULE 40	PVC ID IN	. 1-1/2	LENGTH ET	38.8	TOP ELEV.	27.2
INTAKE ELEMENT: TYPE	NO. 20 SLOTTE	·	i. 1-7/8	LENGTH, FT.		TIP ELEV.	-21.6
			v. 4-1/2 & 3-7/8	-			-57.1
FILTER: MATER	110.20/110					_ = = • • • • • •	
PAY QUANTITIES							
3.5" DIA. DRY SAMPLE BOR	ING LIN. FT.	73.5 NC	O. OF 3" SHELBY T	UBE SAMPLES	3		
3.5" DIA. U-SAMPLE BORING		12 NC	. OF 3" UNDISTUI	RBED SAMPLE	s		5
CORE DRILLING IN ROCK	LIN. FT.	от	HER:				
BORING CONTRACTOR			GEOSERVICE	S CORP.			
DRILLER	STEVE WINGET		HELPERS		JOHN	PERSINGER	<u> </u>
REMARKS	INSTALLED PIE	ZOMETER.					
RESIDENT ENGINEER		MICHAEL WEG	CKLER		DATE	3-4	1-02
·			-		ВО	RING NO.	B-1UP

MUESER RUTLEDGE CONSULTING ENGINEERS BORING LOG

PROJECT:	WASHINGTON MONUMENT VISITOR CENTER
OCATION :	WASHINGTON, D.C.

BORING NO. B-2

SHEET 1 OF 2

FILE NO. 9726B

SURFACE ELEV. 23.5±

RES. ENGR. MICHAEL WECKLER CASING SAMPLE DAILY **REMARKS** STRATA DEPTH BLOWS SAMPLE DESCRIPTION BLOWS/6" PROGRESS NO. DEPTH Grass at surface. 16 Brown silty fine to coarse sand, some 1D 0.0 5-6 07:15 45 * Drilled ahead of gravel, trace brick, vegetation (Fill) (SM) 10-16 2.0 03-11-02 F 52 casing then drove Brown concrete fragments, some fine to 16-69 2D 2.0 Monday 67* casing. coarse sand (Fill) 13-32 4.0 Clear WC=21 Brown fine sandy silt, trace clay, clay seams 5 6* 6-7 3D 4.0 40°F 7* (Fill) (ML) 6-8 6.0 12* Stiff brown clayey silt, some fine sand (ML) T1(A) pp=3.5, WC=20 6-8 4D 6.0 8 19* 11-11 8.0 T2 21* WC=16 Brown silty fine to medium sand, trace clay, 5-10 5D 8.0 20* 10 10-14 fine sandy silt pockets (SM) 10.0 9* Stiff brown silty clay, some fine to medium pp=2.2, WC=17 5-8 10.0 6D sand, trace gravel, fine sand pockets (CL) 10* 12.0 8-11 13* 13* T1 (A) 14* Stiff brown clayey silt, some fine sand (ML) 15 pp=2.3, WC=17 7D 14.0 4-5 30* 15.5 20* 18 31* 35* 20 54* No recovery 19.0 5-10 8NR 65 20.5 8 T2 124 Brown fine to medium sand, some silt, trace 8-8 21.0 9D gravel (SM) 119 Rig chatter from 5 22.5 24 175* 24' to 42.5'. 100* 25 Gray black fine to coarse sandy gravel, 23-29 10D 24.0 115* trace silt, silt pockets (GP-GM) 29 25.5 173* 178* 178* 26* Gray fine to coarse sandy gravel, trace silt 30 11D 29.0 17-22 34* (GP-GM) 25 30.5 315* pp=Pocket **T3** 70* 98* Penetrometer 119* Unconfined Compres-Top: Brown fine to coarse sand, some silt, 35 12D 34.0 15-15 clay pockets, trace gravel, clay (SM-SC) 140* sive Strength in tsf. 31 35.5 200* Bot: Brown red brown gravelly fine to coarse sand, some silt, trace clay pockets 392* WC=Water Content (SM) USED in percent of dry 40 MUD weight. No recovery 100/4" 13NR 39.0 39.3 Brown gravelly fine to coarse sand, some 23-17 41.0 14D 42.5 REC=4" In second silt (SM) 42.5 15 attempt. Medium gray silty clay, trace fine sand (CL) 45 pp=1.0, WC=23 15D 4-4 44.0 45.5 5 T1(D) pp=0.8, WC=21 Medium gray silty clay, trace fine to coarse 16D 48.5 3-4 sand pockets & layers, gravel (CL) 50 50.0 6 14:00 End of Boring at 50'. B-2 **BORING NO.**

MUESER RUTLEDGE CONSULTING ENGINEERS

						BORING NO	D	B-2		
						SHEET	2	_ OF _	2	
PROJECT		WASHIN	WASHINGTON MONUMENT VISITOR CENTER				FILE NO. 9726B			
LOCATION	٧		WASHINGTON, D.C.				ELEV. 23.5±			
BORING L	OCATION	SEE BOR	RING LOCAT	ION PLAN		DATUM		NGSD		
20201050	NUMBER A	ND METHOD	C OF STABI	IZING BODE	HOI E					
BORING EC	QUIPMENT A			<u>IZING BOREI</u>	HOLE					
TYPE OF BO	ORING RIG		OF FEED IG CORING	CAS	SING USED	X YES		NO		
TRUCK	CME-5	-		DIA.	, IN 4	DEPTH, FT. I	FROM	0	TO 40	
SKID		 HYDRA	ULIC	DIA., IN. DEPTH, FT.			FROM		то	
BARGE		OTHER	<u></u>	DIA.	, IN.	DEPTH, FT. 1	FROM		то	
OTHER		_	·							
TYPE AND	SIZE OE:	•		DRILL	ING MUD USE	XYES		NO		
D-SAMPLER		PLIT SPOON			TER OF ROTARY		3-7/8			
U-SAMPLER		EIT OF COIL			OF DRILLING MUD		BENTONITE	, QUIK	- GEL	
S-SAMPLER	 -									
CORE BARRE				AUGE	R USED	YES	X	NO		
CORE BIT				TYPE A	AND DIAMETER, II	N				
DRILL RODS	AW									
				* CASIN	IG HAMMER, LBS		AVERAGE FA		30	
				*SAMPI	ER HAMMER, LB	s. <u>140</u>	AVERAGE FA	ALL, IN.	30	
WATER LEV	VEL OBSER\	/ATIONS IN E				*MECHANICAL	HAMMER U	SED.		
DATE	TIME	DEPTH OF HOLE (FEET)	DEPTH OF CASING (FEET)	DEPTH TO WATER (FEET)		CONDITIONS OF OBSERVATION				
3-11-02	14:00	50	40	23.2	MUD LEVEL UPON COMPLETION.					
3-12-02	5:20	50	40	28.9	OVERNIGHT READING.					
								_		
										
					<u> </u>					
PIEZOMETE	ER INSTALLE	D YE	s X	IO SKET	CH SHOWN O	N				
STANDPIPE:	TYPE			ID, I	N	LENGTH, FT.	TOF	ELEV.		
INTAKE ELEM								TIP ELEV.		
FILTER:	MATER	RIAL								
										
PAY QUAN	<u>TITIES</u>					TUDE DAMES EO				
	SAMPLE BOF				O. OF 3" SHELBY	_				
	AMPLE BORIN		LIN. FT. NO. OF 3" UNDISTU				s		 	
CORE DRILLI	ING IN ROCK	LIN.	F f		THER:					
BORING CO	ONTRACTOR				GEOSERVIC					
DRILLER			E WINGET_		HELPERS		JOHN PERSINGER			
REMARKS		BORI	BOREHOLE WAS GROUTED UPON COMPLETION.						0.00	
RESIDENT I	ENGINEER		MICHAEL WECKLER				DATE		2-02	
	•						BORIN	G NO.	B-2	

MUESER RUTLEDGE CONSULTING ENGINEERS BORING LOG

PROJECT: LOCATION: WASHINGTON MONUMENT VISITOR CENTER

WASHINGTON, D.C.

BORING NO. B-3
SHEET 1 OF 2
FILE NO. 9726B
SURFACE ELEV. 24.1

RES. ENGR.

OSCAR CARPIO

CASING SAMPLE DAILY **REMARKS** STRATA DEPTH BLOWS DEPTH BLOWS/6" SAMPLE DESCRIPTION **PROGRESS** NO. Grass on top of Brown silty fine to medium sand, trace 15 1D 4-8 0.0 07:30 27 surface. gravel, coarse sand, grass (Fill) (SM) 2.0 6-5 02-28-02 F 29 Brown fine to coarse sand, some silt, 4-4 2D 2.0 Thursday gravel, trace brick, vegetation (Fill) (SM) USED 8-14 4.0 Clear Possible wash. Brown gravelly fine to coarse sand, some 5 4-4 3D 4.0 20°F silt, trace shells, clay (Fill) (SM) 6 4 5.5 Brown fine sandy silt, trace clay, clay WC=17 7.0 4-8 4D pockets, lignite (ML) 11 8.5 Brown silty fine sand (SM) 10 5-6 5D 9.0 10.5 8 Brown gray fine sandy silt, trace clay, mica WC=18 4 6D 11.5 (ML) 5-6 13.0 T1(A) 15 WC=20 Stiff brown fine sandy clay, trace mica (CL) 4-6 7D 14.0 15.5 6 20 WC=18 Brown fine sandy silt, trace clay, gravel 8D 19.0 4-4 (ML) 20.5 8 23 25 7-21 Top: Brown silty fine sand, trace gravel, 24.0 9D coarse sand (SM) 27 25.5 Bot: Brown gravelly fine to coarse sand, some silt (SM) Brown fine to coarse sandy gravel, trace silt 30 10D 29.0 30-44 10;30 (GP-GM) 26/3" 30.3 07:30 03-01-02 Friday **T3** Clear 35 Brown fine to medium sand, some silt, 11D 34.0 53-32 30°F cemented sand, trace gravel, coarse sand 35.4 15/5" (SM) pp=Pocket Penetrometer Unconfined Compres-40 sive Strength in tsf. Brown fine to coarse sandy gravel, trace silt 12D 39.0 86-14/3" (GP-GM) 39.7 WC=Water Content in percent of dry weight. Do 12D (GP-GM) 45 13D 44.0 23-18 68 45.5 47 Medium gray silty clay, trace gravel, fine to T1(D) pp=0.5, WC=21 5-5 14D 48.5 50 coarse sand (CL) End of Boring at 5 50.0 13:00 50'. B-3 BORING NO.

MUESER RUTLEDGE CONSULTING ENGINEERS

BORING NO. B-3

						SHEET	2	OF	2	
PROJECT	JECT WASHINGTON MONUMENT VISITOR CENTER		OR CENTER	FILE NO.	9	9726B				
LOCATION		WASHIN	WASHINGTON, D.C.				SURFACE ELEV. 24.1			
BORING L		SEE BO	SEE BORING LOCATION PLAN			DATUM	NGSD			
DOMING L					1	_				
BORING EC	DUIPMENT A	ND METHOD	S OF STABIL	IZING BORE	HOLE					
<u>DOMINO EG</u>	NEIST A									
TYPE OF FEED TYPE OF BORING RIG DURING CORING CASING USED							Γ	NO		
TRUCK	CME-5			DIA.	X YES DEPTH, FT.	_		то 39		
SKID	OIVIL O	HYDRA		DIA.	DEPTH, FT.	_		то		
		OTHER			, IN.	DEPTH, FT.	FROM		то	
BARGE						 ,				
OTHER										
TYPE AND	SIZE OF:			DRILL	ING MUD USEI	X YES	l	NO ,		
D-SAMPLER		LIT SPOON		DIAME	TER OF ROTARY	BIT, IN		3-7/8		
U-SAMPLER				TYPE (OF DRILLING MUE		В	ENTONITE		
S-SAMPLER							_	 -		
CORE BARRI	EL			AUGE	R USED	YES	. [X NO		
CORE BIT				TYPE A	AND DIAMETER, I	N.			····	
DRILL RODS	AW									
				* CASIN	IG HAMMER, LBS		AVERAC	SE FALL, IN.	30	
				*SAMPL	ER HAMMER, LB	s140	AVERAC	SE FALL, IN.	30	
\A/ATED E\	VEL OBSERI	ATIONS IN E	OREHOLE			*MECHANICA	L HAMME	R USED.		
VVATERLE	VEL OBSERV		,	DESTI TO						
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER	CON	NDITIONS OF OF	SERVAT	ION		
DATE	INVIL	(FEET)	(FEET)	(FEET)						
3-1-02	13:30	50	39	17.4	MUD LEVEL UPON COMPLETION.					
3-1-02	10.00									
	l	<u> </u>								
PIEZOMETE	ER INSTALLE	D YE	s X N	IO SKET	TCH SHOWN O	N				
<u> </u>										
STANDPIPE:	TYPE			_ ID, I	N	LENGTH, FT.		TOP ELEV.		
INTAKE ELEN					IN	LENGTH, FT.		TIP ELEV.		
FILTER:					IN	LENGTH, FT.		BOT. ELEV.		
1121211										
PAY QUAN	TITIES						_			
3.5" DIA. DRY	SAMPLE BOF	RING LIN.	FT		O. OF 3" SHELBY		-			
3.5" DIA. U-S/	AMPLE BORIN	G LIN.	FT	N	O. OF 3" UNDIST	URBED SAMPLE	:s			
CORE DRILL	ING IN ROCK	LIN.	FT	°	THER:		-			
BORING CO	ONTRACTOR				GEOSERVIC				_	
DRILLER			'E WINGET	GET HELPERS				PERSINGE	₹	
REMARKS		BORI	BOREHOLE WAS GROUTED UPON COMPLETION.							
RESIDENT	ENGINEER			OSCAR CA	ARPIO		•		1-02	
	•						ВО	RING NO.	B-3	

PROJECT: WASHINGTON MONUMENT VISITOR CENTER LOCATION: WASHINGTON, D.C.

BORING NO. B-4U

SHEET 2 OF 3

FILE NO. 9726B

SURFACE ELEV. 18.8

RES. ENGR. MICHAEL WECKLER CASING SAMPLE DAILY STRATA DEPTH **BLOWS** REMARKS SAMPLE DESCRIPTION DEPTH BLOWS/6" **PROGRESS** NO. USED Cont'd Medium gray silty clay, some fine to REVERT pp=0.8, WC=20 17D 51.5 3-4 03-08-02 medium sand, trace f-m sand pockets (CL) 53.0 Friday Stiff gray silty clay, trace fine to medium pp=1.3, TV=0.7 PUSH=24" 3U 53.5 Clear 55 WC=27 sand (CL) 55.5 REC=23" 60°F Medium gray silty clay, some fine sand (CL) PP=0.8, WC=31 2-3 18D 56.5 58.0 4 60 Stiff gray silty clay, trace fine sand (CL) pp=1.0, WC=35 19D 61.5 1-2 63.0 Stiff gray brown silty clay, some fine sand, pp=2.8, WC=46 PUSH=24" 4U 63.5 65 trace gravel (CH) REC=23" 65.5 Stiff gray silty clay, some fine sand, trace T1(D) pp=1.5, WC=47 2-4 20D 66.5 lianite (CL) 5 68.0 70 Stiff gray silty clay, some fine sand, trace pp=0.9, WC=47 2-3 21D 71.5 pp=1.5, TV=0.6lignite, fine sand seams (CL) 73.0 5 WC=40 Stiff gray silty clay, trace fine sand (CL) PUSH=24" 5U 73.5 75 REC=6" 75.5 pp=2.8, TV=1.0 Stiff gray silt, trace fine sand (MH) PUSH=24" 6U 76.0 WC=49 78.0 REC=22" Top: Stiff gray fine to medium sandy clay, 5-6 22D 78.0 trace gravel, fine sand layers (CL) 80 22D Bot pp=1.0 8 79.5 Bot: Stiff gray clay, trace fine sand, fine WC=45 sand pockets (CH) Top: Stiff gray clay (CH) 23D 83.0 3-6 Bot: Gray micaceous silty fine to medium 85 23D Top pp=1.25 2-5 85.0 sand, trace gravel, rock fragments, gray WC=49 D silty clay pockets (SM) WC=13 Gray micaceous fine to medium sand, some End of Boring at 32-68/5" 24D 88.0 13:15 89.4 silt, trace rock fragments (Decomposed 89.5'. 89.4 Rock) (SM) pp=Pocket Penetrometer Unconfined Compres-95 sive Strength in tsf. TV=Torvane Shear Strength in tsf. 100 WC=Water Content in percent of dry weight. BORING NO. B-4UP

PROJECT: LOCATION: WASHINGTON MONUMENT VISITOR CENTER

WASHINGTON, D.C.

BORING NO. B-4U

SHEET 1 OF 3

FILE NO. 9726B

SURFACE ELEV. 18.8

						ILLO.		WICHAEL WECKLEN
DAILY		SAMPLI	E				CASING	
PROGRESS	NO.	DEPTH	BLOWS/6"	SAMPLE DESCRIPTION	STRATA	DEPTH	BLOWS	REMARKS
09:15	1D	0.0	4-6	Brown fine sandy silt, trace vegetation (Fill)	F		23	Grass at surface.
03-07-02		2.0	8-10	(ML)		2	72]
1 3	2D	2.0	6-6	Brown fine sandy silt (ML)				WC=18
Thursday	20			Brown into carray on (iii_)	i		62	1
Clear		4.0	7-11	Oxist harrow alayes ailt same fine sand (MI)		5	8	pp=3.25, WC=17
60°F	3D	4.0	6-9	Stiff brown clayey silt, some fine sand (ML)		5		pp=3.25, WC=17
		6.0	12-15				20*	
	4D	6.0	8-9	Do 3D (ML)				pp=0.5, WC=24
		8.0	14-15				17*]
	5D	8.0	6-10	Brown red brown fine sandy silt, trace clay			22*	pp=2.8, WC=18
		10.0		(ML)		10	23*	
!	6D	10.0	7-9	Do 5D (ML)			12*	pp=3.75, WC=19
	-00	12.0	11-14	(···-)	T1(A)		13*	1''
	70			Brown fine sandy silt, trace gravel, mica	(,			pp=1.5, WC=18
	7D	12.5		(ML)			16*	pp=1:0, 11 0=10
		14.0				15		nn-1 5 TV-0 4
, [,	1U	14.0		Brown fine sandy silt,trace clay, gravel (ML)		13		pp=1.5, TV=0.4 WC=17
		16.0	REC=11"					-
	8D	16.0	5-5	Brown fine sandy silt, trace clay, gravel				pp=1.5, WC=19
		18.0	7	(ML)			19*	1
						18.5	24*	*Drilled ahead of
	9D	19.0	5-5	Brown fine to medium sand, some silt (SM)	T2	20	30*	casing and then drove
		20.5	6		·		50*	casing.
		20.0				21.5	52*	
				·			167*	
				·		-	185*	1
	405	04.0	00.04	Brown fine to coarse sandy gravel, some		25	110*	1
, ,	10D		36-34	· -			100*	†
		25.5	30	silt (GM)				
·							93*	-
							135*	-
							115*	4
1	11D	29.0	17-14	Do 10D (GM)		30	75*	4
'		30.5	12		Т3		75*	
	12D	31.0	23-23	Brown fine to coarse sandy gravel, some			79*]
		32.5	41	clay (GM-GC)			132*	_
	-						180*	
	13D	34.0	100/6"	Brown fine to coarse sandy gravel, some		35	1	
	100	34.5	100/0	silt, trace clay (GM)				
		34.3		,, (, (,)			210*	1 !
							374*	_
.		_					USED	-
		0.5	44.5	Brown fine to coorse conductrovel trace cilt		40	+	.
	14D		11-9	Brown fine to coarse sandy gravel, trace silt		40	REVERT	-
		41.5	9	((GP-GM)		44 -	+ +-	14 WC 00
	15D	41.5	3-4	Stiff gray silty clay, trace fine sand (CL)		41.5	+	_pp=1.4, WC=23
		43.0	4				<u> </u>	1
	2U	43.5	PUSH=24"	Soft gray silty clay, trace fine sand, gravel				_pp=0.4, WC=23
14:30		45.5	REC=6"	(CL)		45		
15:30	16D	46.5	2-3	Medium gray silty clay, trace fine sand, fine	T1(D)			pp=0.5, WC=22
1		48.0	3	sand pockets (CL)	` ´]
03-08-02	-	70.0		F -,,/	}		 	7
Friday	 					50	 	
Clear	-					135	+ + +	-
60°F	<u> </u>	-				-	+	
L	<u> </u>	L	<u> </u>		J	ROP	ING NO	B-4U
						DUR	ING NO	, <u> </u>

BORING NO. B-4U

						SHEET	3	OF _	3	
PROJECT		WASHIN	GTON MONU	MENT VISITO	OR CENTER	FILE NO.	972			
LOCATION		WASHIN	GTON, D.C.			SURFACE I	ELEV	18.8	3	
BORING L		SEE BOF	RING LOCATI	ON PLAN		DATUM		NGSD		
BORING EQ TYPE OF BOTRUCK SKID BARGE OTHER TYPE AND S D-SAMPLER U-SAMPLER S-SAMPLER	DRING RIG CME-58 SIZE OF: 2" O.D. SF 3" SHELBY	ND METHOD TYPE DURIN MECHA HYDRA OTHER	S OF STABIL OF FEED IG CORING NICAL ULIC	IZING BOREH CAS DIA. DIA. DIA. DRILL DIAME	HOLE SING USED , IN 4 , IN. , IN. LING MUD USEI TER OF ROTARY OF DRILLING MUD R USED	X YES DEPTH, FT. I DEPTH, FT. I DEPTH, FT. I	FROM	NO 0 0 NO 3-7/8	то <u>4</u> то <u> </u> то <u> </u>	11.5
					AND DIAMETER, I		<u> </u>	_		
DRILL RODS WATER LEV	AW	/ATIONS IN E		IG HAMMER, LBS LER HAMMER, LB	i. <u>140</u>	AVERAGE AVERAGE - HAMMER	FALL, IN.	30 30	_	
DATE	TIME	DEPTH OF HOLE (FEET)	DEPTH OF CASING (FEET)	DEPTH TO WATER (FEET)	CO	NDITIONS OF OE	SERVATIO	N		
3-8-02	13:00	89.5	41.5	11.9	MUD LEVEL	UPON COMPI	LETION.			
3-11-02	5:30	89.5	41.5	22.6	MUD LEVEL	AFTER WEEK	END.			
						· · ·		•		
PIEZOMETE	ER INSTALLE	<u>D</u> YE	s X	IO SKET	rch shown o	N			<u> </u>	
STANDPIPE:	TYPE			1D, I	N	LENGTH, FT.	T	OP ELEV		_
INTAKE ELEN				OD.	IN	LENGTH, FT.	T	IP ELEV.		
FILTER:	MATER	RIAL		OD,	IN	LENGTH, FT.	В	OT. ELEV		
PAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING LIN. FT7				12 N	O. OF 3" SHELBY O. OF 3" UNDIST THER:		-		ļ	
					GEOSERVIC	ES CORP.				
DRILLER			E WINGET		}	JOHN PE	RSINGER	}		
REMARKS										
RESIDENT	ENGINEER		BOREHOLE WAS GROUTED UPON COMPLETION. MICHAEL WECKLER				DATE	3-7	7-02	
,,,,,,							BOR	ING NO.	B-	4U

PROJECT:	
LOCATION	,

WASHINGTON MONUMENT VISITOR CENTER

WASHINGTON, D.C.

BORING NO. B-5

SHEET 1 OF 2

FILE NO. 9726B

SURFACE ELEV. 25.0

	DAILY		SAMPLI	E				CASING	DEMARKS
l	PROGRESS	NO.	DEPTH	BLOWS/6"	SAMPLE DESCRIPTION	STRATA	DEPTH	BLOWS	REMARKS
Γ	09:00	1D	0.0	7-8	Brown silty fine to medium sand, trace				Grass at surface.
	03-04-02		2.0	9-12	gravel, grass, clay (Fill) (SM)	F		67	
	Monday	2D	2.0	33-40	Brown fine o medium sand, some silt, trace			72	REC=6"
	Clear		3.3	27/3"	gravel, vegetation, shells, mica (Fill) (SM)		3.3	55	* Drilled ahead of
	30°F	3D	4.0	7-5	Brown fine sandy silt, trace clay, lignite		5	11*	casing and then
	30 1	0.0	6.0	5-5	(ML)	'		12*	drove casing.
		4D	6.0	3-4	Do 3D (ML)				WC=16
		40		4-4	Do 05 ()		8	15*	
			8.0		Top 12": Stiff gry brn si cl, tr f-m sand (CL)				5D top pp=3.7,
		5D	8.0	5-8	Bot: Brown fine sandy silt, trace clay (ML)		10		WC=18
╸			10.0	12-18			10		WC=16
		6D	10.0	8-9	Brown fine sandy silt, trace clay (ML)	T4/4\			WC=10
			12.0	11-17		T1(A)		13*	-
								10*	_
-								10*	
_		7D	14.0	6-8	Gray silt, some fine sand, trace clay, clay		15	_	pp=4.25, WC=18
II.			15.5	11	pockets (ML)	ĺ		11*] !
8							į	12*	<u>.</u>
								10*	
			-					11*	1
		8D	19.0	6-6	Brown fine sandy silt, trace clay, clay		20	16*	WC=16
		<u>0</u> D	20.5	7	pockets (ML)		21	14*	1
_			20.5	,	pockets (WE)			22*	1
								23*	-
							-	35*	1
					The first transfer and		<u> </u>		1
		9D	24.0	13-13	Reddish brown silty fine to medium sand,	,	25	51*	·
			25.5	12	some gravel, trace coarse sand (SM)			39*	1
╸╽					·				1
								ļ	<u> </u>
?						ļ		145*	
		10D	29.0	35-35	Brown fine to coarse sandy gravel, some	T3	30	USED	1
_			30.4	30/5"	silt (GM)			MUD	Rig chatter from
_			00.1			1			31' to 34'.
									j
		-							
		110	04.0	50.07	Brown fine to coarse sandy gravel, trace	1	3.5		1
n i		טוו	34.0	50-37	silt, clay pockets (GP-GM)		- 55		Rig chatter from
			35.3	13/3"	Sitt, ciay pockets (dr-divi)			 	37' to 39'.
-							<u> </u>	+	31 10 38.
								+ +-	Dim abattas from
								+	Rig chatter from
		12D	39.0	100/6"	Brown fine to coarse sandy gravel, some		40	 	40' to 42'.
			39.5		silt (GM)	ļ			pp=Pocket
								<u> </u>	Penetrometer
				1			43		Unconfined Compres-
		<u> </u>							sive Strength in tsf.
		13D	44.0	17-17	Stiff gray silty clay (CL)		45		pp=1.4, WC=22
		130		•	July 2007 (3-7)				1,,
		<u> </u>	45.5	9		T1(D)			WC=Water Content
		<u> </u>				1 1(0)		+	in percent of dry
Ì					0 ()	1	}	+	-
		14D	48.5	3-2	Soft gray silty clay, trace fine sand (CL)		<u> </u>	v	weight.
	14:30		50.0	4	·	ļ	50	-	14D pp=0.3, WC=23
Γ								1	End of boring at 50'.
			·			J	<u></u>		L
ן י							BOF	RING NO). <u>B-5</u>

BORING NO. B-5

			SHEET	2	OF _	2				
PROJECT		WASHIN	GTON MONL	MENT VISITO	OR CENTER	FILE NO.	972			
LOCATION	J	WASHIN	GTON, D.C.			SURFACE E	ELEV	25.0)	
BORING L	OCATION	SEE BOF	RING LOCATI	ON PLAN		DATUM _		NGSD		
BORING EC	QUIPMENT A	ND METHOD	S OF STABIL	IZING BOREH	<u>IOLE</u>					
		TYPE	OF FEED							
TYPE OF BO	ORING RIG	–	G CORING	CAS	SING USED	X YES		МО		
TRUCK	CME-5	5 MECHA	NICAL	DIA.,	, IN <u>4</u>	DEPTH, FT. F	FROM	0	то _	29
SKID		HYDRA	ULIC	DIA.,	, IN	_ DEPTH, FT. F	FROM		то _	
BARGE		OTHER		DIA.,	, IN	DEPTH, FT. F	FROM		то _	
OTHER										
TYPE AND	SIZE OF:			DRILL	ING MUD USED	XYES		NO		
D-SAMPLER	2" O.D. SF	LIT SPOON		DIAME	TER OF ROTARY	BIT, IN.		3-7/8		
U-SAMPLER				TYPE C	OF DRILLING MUD		BEN	TONITE		
S-SAMPLER							<u> </u>	1		
CORE BARRE	EL			AUGE	R USED	YES	X] ио		
CORE BIT				TYPE A	ND DIAMETER, IN	۷				
DRILL RODS	AW									
				•	IG HAMMER, LBS. .ER HAMMER, LB\$		AVERAGE I			
\444TED E\	/EL ODGED\	ATIONS IN B	ODENOLE			*MECHANICAL	HAMMER U	JSED.		
WATER LEV	EL OBSEK	ATIONS IN B			Γ					 -1
DATE	TIME	DEPTH OF HOLE (FEET)	DEPTH OF CASING (FEET)	DEPTH TO WATER (FEET)	COV	IDITIONS OF OB	SERVATION	1		
3-4-02	14:20	50	29	11.4	MUD LEVEL	UPON COMPL	ETION.			
3-5-02	6:30	50	29	14.6	OVERNIGHT					
PIEZOMETE	ER INSTALLE	ED YE	s X N	IO SKET	CH SHOWN ON	١				
STANDPIPE:	TYPE			ID, I	N	LENGTH, FT.	TC	P ELEV		
INTAKE ELEM				00.1	IN	LENGTH, FT.	TII	P ELEV		
FILTER:	MATER				IN	LENGTH, FT.	ВС	OT. ELEV.	_	
PAY QUAN			FT 1	50 N	O. OF 3" SHELBY	TURE SAMPLES	:			
	SAMPLE BOF									
3.5" DIA. U-SAMPLE BORING LIN. FT. CORE DRILLING IN ROCK LIN. FT.				NO. OF 3" UNDISTURBED SAMPLES OTHER:						
CORE DRILLI	NG IN ROCK	LIN.	F1		IREK.					
00000000		•			GEOSERVICE	S CORP				
	NTRACTOR		E WINGET		HELPERS		JOHN PF	RSINGER		
DRILLER				GROUTED III	PON COMPLET		JOHN PERSINGER			
REMARKS		BORE		MICHAEL WE	DATE 3-5-02					
RESIDENT I	FINGINEEK			WIIOTIALL VVE	-VI\L-I\		-	NG NO.		3-5
							501(1)			

PROJECT:	
LOCATION	

WASHINGTON MONUMENT VISITOR CENTER

WASHINGTON, D.C.

BORING NO. B-6P

SHEET 1 OF 3

FILE NO. 9726B

SURFACE ELEV. 21.7

						nL.	. LINGI.	WICHAEL WECKLER
DAILY		SAMPLI	E				CASING	
PROGRESS	NO.	DEPTH	BLOWS/6"	SAMPLE DESCRIPTION	STRATA	DEPTH	BLOWS	REMARKS
	1D	0.0	3-4	Brown silty fine to medium sand, trace			14	Grass at surface.
07:30	10		5-5	brick, vegetation (Fill) (SM)			31	*Drilled ahead of
03-14-02		2.0		Brown silty fine sand, trace white rock	F			casing and then
Overcast	2D	2.0	6-6		Г			1
Morning		4.0	10-12	fragments (Fill) (SM)				drove casing.
Clear	ЗD	4.0	7-8	Top: Brn f sandy silt, tr clay, veg (Fill) (ML)		5	PUSH	
Afternoon		6.0	8-9	Bot: Stiff brn cl silt, tr fine sand (ML)			17.*	3D Bot: pp=3.5
60°F	4D	6.0	4-7	Brown fine sandy silt, trace clay (ML)			17*	WC=19
00.		8.0	9-13				16*	4D pp=4.0, WC=17
1	<u> </u>		6-10	Brown red brown fine sandy silt, trace clay				5D WC=15
]	5D	8.0		(ML)		10	21*	
		10.0	10-11		٠ .	- 10		0.9 WC-17
	6D	10.0	3-5	Stiff brown red brown silty clay, some fine				pp=2.8, WC=17
		12.0	7-8	sand (CL)			14*	
					T1(A)		14*	
							11*	
	7D	14.0	3-4	Stiff brown silty clay, trace fine sand (CL)		15	16*	pp=2.3, WC=18
<u> </u>	, 0	15.5	6		'		12*	[]
		13.3	J		E		8*	1
							6*	
							5*	
	8D	19.0	3-2	Medium stiff brown clayey silt, some fine		20		WC=18
1		20.5	3	sand (ML)	[[8*	ļ
						22	7*	
			4 .				18*]
		-			T2		18*	pp=Pocket
				Brown silty fine to medium sand, trace		25		Penetrometer
	9D	24.0	5-6	1	İ			•
		25.5	10	gravel (SM)		25.5		Unconfined Compres-
								sive Strength in tsf.
							138*]
						-	266*	WC=Water Content
	10D	29.0	20-18	Brown coarse to fine sandy gravel, some		30	87/6"	in percent of dry
	100		37	silt (GM)			USED	weight.
		30.5	37	Sitt (Civi)			MUD	
							MOD	
						<u> </u>		-
								_
ļ	11D	34.0	21-18	Gray white brown coarse to fine sandy	T3	35		1
		35.5	16	gravel, some silt, trace black rock	ļ	İ		
1				fragments (GM)	İ]
								1
							 	Rig chatter at 25.7'
		<u> </u>		Description and some group silt	ļ	4.0		to 39'.
	12D		24-23	Brown coarse to fine sand, some gravel, silt	1	40	 	10 39.
		40.5	22	(SM)			<u> </u>	
								_
					1	43		
	<u> </u>]
	100	44.0	0.4	Stiff gray silty clay, trace fine sand, lignite,		45		pp=1.0, WC=23
	13D		2-4			 73	+ +	Jpp-1.0, **O-20
		45.5	4	gravel (CL)			+ + -	4
	L	<u></u>			T1(D)		+	4
								_
İ	14D	48.5	2-2	Medium gray silty clay, some fine sand (CL)		1_	₩	pp=0.7, WC=20
10.00	· · · ·	50.0	3			50		7
13:00		30.0	,					End of Boring at
						-	1	_
L	L	L	L			POF	ING NO	50'. B-6P
						DUF	IIIA NO	D-01

SHEET_2	_0F <u>3</u> _
FILE NO.	9726B
SUBCODE	

PIEZOMETER RECORD

PROJECT LOCATIO PIE ZOME SEE	N <u>WA</u> TER	LOC	<i>(۲۵۲)</i> OITA	N _		NT V	ISITOR	[•	NO. B-6P ISTALLATION 3/14/02				
ST	RATA	PIE Z	ZOMET ALLAT ETAILS	ER ION	DEPTH (FT)	<u></u>	PIEZOMETER TYPE SLOTTED PVC INTAKE POINT depth to bottom, ft= 50							
SUF ELEV	OUND RFACE	#65 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		7000 8000 8000 8000	O TOP OF SAUG	(PELLETS)	depth to top, $ft = \frac{8}{42}$ length, $ft = \frac{42}{42}$ diameter, in = $\frac{42}{42}$, $ft = \frac{1}{42}$							
	(A) 72 73 73 7(D)	からからいっているというとうないというというというのであるというというというというというというというというというというというというというと		(61	30 TOP OF **AEEN=35 "+0 PIEZ. BOT-45	3/15/03 3/15/03 3/15/03 SIRIOS	CLOCK 14:15 6:15 12:43 12:50 8:00	DEPTH - RIM TO WATER 4.00 27.15 27.15 27.15 27.15 27.8	ELEVATION OF WATER 17.5 - S.65 - 5.65 - 5.65 - G.1	REMARKS TUITIAL READING OVERVIGHT READING - 24 UR READING READING ATTER REING HEAD TIST				
			TTOM	OF										

Sand Bentonite

A A Gravel Grout

GROUND SURFACE ELEV . 21.7

PIEZOMETER NO. 8-69

						BORING N	0	B-6P	
						SHEET	3	OF	3
PROJECT		WASHIN	GTON MONU	JMENT VISIT	OR CENTER	FILE NO.			
LOCATION	N	WASHIN	GTON, D.C.			SURFACE	ELEV.	21	.7
BORING L	OCATION	SEE BO	RING LOCAT	ION PLAN		DATUM		NGSE)
BORING EC	QUIPMENT A	ND METHOD	S OF STABIL	<u>IZING BORE</u>	<u>HOLE</u>				
TYPE OF B	ORING RIG CME-5	DURI		DIA	SING USED ., IN 4	X YES _ DEPTH, FT. DEPTH, FT.	FROM	NO 0	TO 29.5
BARGE		OTHER			., IN.	DEPTH, FT.			то
OTHER									
TYPE AND D-SAMPLER	2" O.D. SF	PLIT SPOON		DIAME	LING MUD USED TER OF ROTARY OF DRILLING MUD	BIT, IN.		NO 3-7/8 REVERT	
S-SAMPLER								「 ▽]	
CORE BARRI	EL				RUSED	YES		X NO	
CORE BIT				TYPE /	AND DIAMETER, IN	N			
DRILL RODS	<u>AW</u>					4.40	41/504		00
					NG HAMMER, LBS.			GE FALL, IN.	30
				*SAMPI	LER HAMMER, LBS	s. <u>140</u>	AVERA	GE FALL, IN.	
WATER LEV	VEL OBSERV	ATIONS IN E	OREHOLE			*MECHANICAI	L HAMM	ER USED.	
DATE	TIME	DEPTH OF HOLE (FEET)	DEPTH OF CASING (FEET)	DEPTH TO WATER (FEET)	CON	IDITIONS OF OE	SERVA	TION	
3-14-02	13:10	50	29	2.2	MUD LEVEL	UPON COMPI	LETION	1.	
				_					
PIEZOMETE	ER INSTALLE	ED X YE	s N	IO SKET	TCH SHOWN ON	N	SH	HEET 2 OF 3	l .
STANDPIPE:	TYPE	SCH	IEDULE 40 P	VC ID. I	N. 1-1/2	LENGTH, FT.	34.8	TOP ELEV.	21.5
INTAKE ELEN			0 SLOTTED		IN. 1-7/8	LENGTH, FT.			-23.3
FILTER:	MATEF	-	NO. 2 SAND		IN. 4-1/2 & 3-7/8			BOT. ELEV.	-28.3
PAY QUAN					O. OF 3" SHELBY	_			
	AMPLE BORING			N	O. OF 3" UNDISTU	JRBED SAMPLE	s		
CORE DRILLI		LIN.			THER:				
						-c copp			
	ONTRACTOR				GEOSERVICE	-5 CORP.	101 111	DEDOUGE	
DRILLER			E WINGET		HELPERS		JOHN	PERSINGE	<u> </u>
REMARKS		PIEZ	DMETER INS		-014 55		DATE		14.00
RESIDENT	ENGINEER	-		MICHAEL WE	CKLEH	* * * * * * * * * * * * * * * * * * * *	DATE		14-02 D.6D
							RC	PRING NO.	B-6P

PROJECT: LOCATION: WASHINGTON MONUMENT VISITOR CENTER

WASHINGTON, D.C.

BORING NO. B-7
SHEET 1 OF 2
FILE NO. 9726B
SURFACE ELEV. 19.8

DAILY		SAMPLE	=				CASING	
PROGRESS	NO.	DEPTH	BLOWS/6"	SAMPLE DESCRIPTION	STRATA	DEPTH	BLOWS	REMARKS
07:00	1D	0.0	3-4	Brown silty fine to medium sand, some	F		13	
03-12-02	-,-	2.0	5-5	gravel, trace coarse sand, vegetation (Fill)		2	23	
Tuesday	2D	2.0	5-4	Stiff brown fine sandy silt, trace clay (ML)			25	pp=3.8, WC=16
Overcast		4.0	8-11				65*	
50°F	3D	4.0	6-9	Brown silty clay, some fine sand, trace		5		
30.	0.0	6.0	9-11	gravel (CL)			5*	
	4D	6.0	5-7	Very stiff brown clayey silt, some fine sand			25*	pp=2.9, WC=19
		8.0	10-10	(ML)			21*	
	5D	8.0	5-7	Do 4D, trace gray clay pockets (ML)			23*	pp=3.3, WC=19
		10.0	11-15		T1(A)	10	25*	
1	6NR	10.0	8-9	No recovery	, ,		20*	
	01111	12.0	11-15				22*	
	7D	12.0	7-6	Brown fine sandy silt, trace clay, clay				WC=17
	75	14.0	8-32	pockets, gravel (ML)			18*	
	8D	14.0	6-6	Brown fine sandy silt, trace clay, gravel		15		WC=19
	0.0	15.5	9	(ML)	'		85*	*Drilled ahead of
		13.3	3	\``` - '		16.5		casing and then
								drove casing.
							120*	
	9D	19.0	16-19	Gray black brown fine to coarse sandy		20	117*	
	90		27	gravel, some silt (GM)			240*	†
	-	20.5	21	graver, some sitt (divi)				Possible boulder
	40010	00.0	100/1"	No recovery				from 23' to 24'.
11	TUNH	23.0	100/1"	TWO TECOVERY			570*	110111 20 10 24.
13:00	10	23.1	DEC 400/	Gray hard gneiss; boulder		25	225*	
11:00	1C	23.1	REC=42%	Gray Hard griess, bodider	Т3	23	52*	
03-13-02		24.0	00.05	Brown fine to coarse sandy gravel, some	13		40*	
Wednesday	טוו	24.0	23-25	silt (GM)		ļ	145*	
Rain		25.5	21	Sit (Givi)			132*	
Overnight			00.00	Brown gravelly coarse to fine sand, some		30	188*	
	12D	29.0	29-29	I = :		30	132*	-
		30.5	30	silt (SM)				pp=Pocket
								Penetrometer
								Unconfined Compres-
	4.5-		46 66/68	Gray brown coaree to fine candy grayel		35		sive Strength in tsf.
1	130		45-55/5"	Gray brown coarse to fine sandy gravel, some silt (GM)		33	117*	SIVE OUTSIIGHT III ISI.
		34.9		Some siit (Givi)				WC=Water Content
'	<u> </u>					38		in percent of dry
				·		30	1	weight.
			0.0	Soft gray silty clay, some gravel, fine sand		40	,	pp=0.4, WC=19
1	14D	39.0	8-6			40	USED	pp=0.4, WC=18
14:30	<u> </u>	40.5	6	(CL)				-
17:30							MUD	-
03-14-02					T4/5:	ļ		-
Thursday				Costs are a citizen along the cond (CL)	T1(D)	4 5	-	0.4 WC 02
Overcast	15D	_	3-4	Soft gray silty clay, trace fine sand (CL)		45	-	pp=0.4, WC=23
Morning		45.5	4				+	4
50°F						ļ	+	-
				10 1 10 10 10 10 10 10 10 10 10 10 10 10			 	140.00
	16D_	48.5	2-2	Medium gray silty clay, trace fine sand (CL)		<u>-</u>	V	WC=22
18:45		50.0	4			50	ļ	End of Boring at 50'.
					1		 	-
L					<u></u>	POR	ING NO.	B-7
,						DUK	ING NU.	

BORING NO. B-7

						SHEET	2	OF2		
PROJECT		WASHIN	GTON MONL	MENT VISITO	OR CENTER	FILE NO.		9726B		
LOCATION	٧	WASHIN	GTON, D.C.			SURFACE	ELEV.	19	.8	
BORING L	OCATION	SEE BOF	RING LOCAT	ION PLAN		DATUM		NGSD		
BORING EC	QUIPMENT A	ND METHOD	S OF STABIL	IZING BORE	HOLE					
-		TYPE	OF FEED							
TYPE OF B	ORING RIG		IG CORING	CA	SING USED	X YES		NO		
TRUCK	CME-5	5 _ MECHA	NICAL	DIA.	, IN <u>4</u>	DEPTH, FT.	FROM _	0	то _	39
SKID		HYDRA	ULIC	X DIA.	, IN	DEPTH, FT.	FROM		то _	
BARGE		OTHER		DIA.	, IN	DEPTH, FT.	FROM _		то _	
OTHER										
						. 				
TYPE AND					ING MUD USED		ļ	NO NO		
D-SAMPLER	2" O.D. SF	PLIT SPOON			TER OF ROTARY	· —		3-7/8		
U-SAMPLER				TYPE	OF DRILLING MUD		BENTO	NITE, QUIK	- GEL	
S-SAMPLER								V		
CORE BARRI	EL NX DOUB				R USED	YES	5 <u>[</u>	Х ио		
CORE BIT	NX DIAMO	DND		TYPE A	AND DIAMETER, IN	l.				
DRILL RODS	AW									
					G HAMMER, LBS.			SE FALL, IN.	30	_
				*SAMPL	ER HAMMER, LBS	s. <u>140</u>	AVERA	GE FALL, IN.	30	
WATER LEV	/EL OBSERV	ATIONS IN B	OREHOLE			*MECHANICA	L HAMME	ER USED.		
		DEPTH OF	DEPTH OF	DEPTH TO						
DATE	TIME	HOLE	CASING	WATER	BSERVAT	ION				
		(FEET)	(FEET)	(FEET)						
3-13-02	9:00	23	23	6	OVERNIGHT	READING.	<u> </u>			
3-13-02	14:25	40. <u>5</u>	39	4.6	END OF DAY	READING.				
3-14-02	15:20_	40.5	39	5	OVERNIGHT	READING.				
3-14-02	16:15	50	39	5.3	MUD LEVEL	UPON COMP	LETION			
			F							
<u>PIEZOMETE</u>	R INSTALLE	<u>D</u> YE:	s X N	IO SKET	CH SHOWN ON					
STANDPIPE:	TYPE				N					
INTAKE ELEN	MENT: TYPE	 		OD, I	IN		***			
FILTER:	MATER	RIAL		OD, I	IN	_ LENGTH, FT.		BOT. ELEV.		
501/61/41/5	ELTIE O									
PAY QUAN			FT 41	D 4 N/	O. OF 3" SHELBY	TIIDE CAMDI E	e			
	SAMPLE BOR						-			
	AMPLE BORIN				O. OF 3" UNDISTU	IRBED SAMPLE				
CORE DRILLI	NG IN BOULD	ER LIN.	FT0	0.9	THER:		-			
						e conn				
	NTRACTOR				GEOSERVICE	:0 CORP.	IOHNI	DEDOMOCE		
DRILLER			E WINGET	000'TED''	HELPERS		JOHN	PERSINGER		
REMARKS BOREHOLE WAS GROU					····	ION.	DATE	2.4	4.02	
RESIDENT I	ENGINEER			MICHAEL WE	CKLER		DATE		4-02	. 7
							RO	RING NO.		3-7

PROJECT: LOCATION: WASHINGTON MONUMENT GROUND IMPROVEMENTS

WASHINGTON, D.C.

BORING NO. W-1
SHEET 1 OF 2
FILE NO. 9726A
SURFACE ELEV. 21.6

							RES	<u>ENGR.</u>	MICHAEL WECKLER
DAIL	V		SAMPL	F				CASING	
	г			BLOWS/6"	SAMPLE DESCRIPTION	STRATA	DEPTH	BLOWS	REMARKS
PROGE		NO.	DEPTH		Top: Brn f-c sand, sm silt, tr veg (Fill) (SM)	=		AUGER	Grass at surface.
07:0		1D	0.0	3-28	Bot 6": Gray gravel, sm f-m sand (Fill) (GP)	F	 		T. 255 G. Odilavo.
03-07			2.0	41-11				- -	WC=19
Thurs	day [2D	2.0	12-20	Brown fine sandy silt, trace clay, vegetation				VVC=19
Clea	ar [4.0	9-6	(Fill) (ML)		4	 	
60°	F [3D	4.0	4-6	Brown fine sandy silt, trace clay (ML)		5	ACCORDO CO CO CO CO CO CO CO CO CO CO CO CO CO	WC=17
	Ĩ	<u> </u>	6.0	10-13]
	ľ	4D	6.0	5-9	Do 3D (ML)				WC=18
	ŀ		8.0	13-16		T1(A)			
)	}	5D	8.0	7-11	Do 3D (ML)	` ′			WC=20
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	-	6D	10.0	7-9	Do 3D (ML)		4.2		pp-3.2 44 0-20
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PROJECT WASHINGTON MONUMENT GROUND IMPROVEMENTS FILE NO. 9726 A							BORING NO	D	W-1	
DOCATION							SHEET		_ OF	2
BORING LOCATION SEE BORING LOCATION PLAN DATUM NGSD BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF BORING RIG TYPE OF FEED DURING CORING CASING USED YES X NO SIZE OF: DATE OF BORING RIG DIA, IN. DEPTH, FT. FROM TO DIA, IN. DEPTH, FT. TOP ELEV. DEPTH DEPT	PROJECT	WASH	INGTON MON	IUMENT GRO	DUND IMPRO	VEMENTS	_			
BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF BORING RIG TYPE OF FEED DURING CORING TRUCK CME-45 MECHANICAL DIA, IN. DEPTH, FT. FROM TO DIA, IN. DEPTH, FT. FROM TO DIA, IN. DEPTH, FT. FROM TO DIA, IN. DEPTH, FT. FROM TO DIA, IN. DEPTH, FT. FROM TO DIA, IN. DEPTH, FT. FROM TO DIA, IN. DEPTH, FT. FROM TO DIA, IN. DEPTH, FT. FROM TO DIA, IN. DEPTH, FT. FROM TO DIA, IN. DEPTH, FT. FROM TO DIA, IN. DEPTH, FT. FROM TO DEPTH, FT. TOP ELEV. THE DEPTH OF DEPTH OF DEPTH OF DEPTH TO DEPTH OF DEPTH OF DEPTH OF DEPTH OF DEPTH TO DEPTH OF DEPTH OF DEPTH OF DEPTH OF DEPTH TO DEPTH OF DEPT								ELEV		
TYPE OF BORING RIG	BORING LO	CATION	SEE BO	RING LOCAT	ION PLAN		DATUM _		NGSD	
TYPE OF BORING RIG										
TYPE OF BORING RIG	DODING FOL	UDMENT A	ND METHOD	S OF STARII	IZING BORE	-OLF				
TYPE OF BORING RIG.	BURING EQU				JANO DONE	1045				
TRUCK	TYPE OF BO	RING RIG			CAS	SING USED	YES	X	NO	
SKID							DEPTH, FT. I		то	o
DIA., IN. DEPTH, FT. FROM TO								FROM	то	o
TYPE AND SIZE OF: D-SAMPLER U-SAMPLER S-SAMPLER CORE BARREL CORE BIT DEPTH OF DEPTH OF DEPTH OF HOLE CASING HAMMER, LBS. AVERAGE FALL, IN. 30 WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE CASING WATER (FEET) FIET) NO OBSERVATIONS MADE. PIEZOMETER INSTALLED PIEZOMETER INSTALLED PAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING 3.5" DIA. DRY SAMPLE BORING 3.5" DIA. JASAMPLE BORING CORE DRILLING TIME DORING CONTRACTOR RONALD STIDHAM BORING CONTRACTOR RONALD STIDHAM BORENDER BORING CONTRACTOR RONALD STIDHAM BORENDER MICHAEL WAS GROUTED UPON COMPLETION. DATE DATE DRILLING MUD USED YES X NO DIAMETER OF ROTARY BIT, IN. AVERACY BY HOLLOW STEM O.D. 6" AUGER USED X YES NO HOLLOW STEM O.D. 6" AVERAGE FALL, IN. 30 AVERAGE FALL, IN. 30 WATER (FEET) NO OBSERVATIONS OF OBSERVATION FILE DATE DO, IN. LENGTH, FT. TOP ELEV. DO, IN. LENGTH, FT. TIP ELEV. DO, IN. LENGTH, FT. TIP ELEV. OO, IN. LENGTH, FT. TIP ELEV. OTHER: BOT ELEV. BORING CONTRACTOR RONALD STIDHAM HELPERS BILLY LUTTRELL BORIEGE MICHAEL WESCREER DATE 3-7-02			OTHER				DEPTH, FT. I	FROM	то	o
D-SAMPLER 2" O.D. SPLIT SPOON DIAMETER OF ROTARY BIT, IN.										
D-SAMPLER 2" O.D. SPLIT SPOON DIAMETER OF ROTARY BIT, IN.						NO MIC HOT	, [],,		NO	
U-SAMPLER	TYPE AND S								NO	
S-SAMPLER CORE BARREL CORE BARREL CORE BIT DRILL RODS AW CASING HAMMER, LBS. AVERAGE FALL, IN. 30 WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME HOLE CASING WATER CONDITIONS OF OBSERVATION (FEET) NO OBSERVATIONS MADE. PIEZOMETER INSTALLED YES X NO SKETCH SHOWN ON STANDPIPE: TYPE 1D, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER: MATERIAL OD, IN. LENGTH, FT. TIP ELEV. FILTER: MATERIAL DATE DAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING 3.5" DIA. DRY SAMPLE BORING CORE DRILLING IN ROCK BORING CONTRACTOR REMARKS BOREHOLE WAS GROUTED UPON COMPLETION. BORENDE CORE DATE AVERAGE FALL, IN. AVERAGE FALL, IN. 30 HOLLOW STEM O.D. 6"		2" O.D. SF	LIT SPOON	 .			-	·		
CORE BARREL CORE BIT DRILL RODS AW CASING HAMMER, LBS. SAMPLER HAMMER,	• • • • • • • • • • • • • • • • • • • •				TYPE	OF DRILLING MUD	·			
CORE BIT TYPE AND DIAMETER, IN. HOLLOW STEM O. D. 6" DRILL RODS AW CASING HAMMER, LBS. AVERAGE FALL, IN. 30 WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME HOLE CASING WATER (FEET) CONDITIONS OF OBSERVATION FEET OD, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. PAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING LIN. FT. 0. O. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. DRY SAMPLE BORING LIN. FT. OTHER: BORING CONTRACTOR RONALD STIDHAM HELPERS BILLY LUTTRELL BORENDE WAS GROUTED UPON COMPLETION. RESIDENT ENGINEER MICHAEL WECKLER MICHAEL WECKLER DATE 3.7-02					ALICE	E LISED	X VES		NO	
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WATER LEVEL OBSERVATIONS IN BOREHOLE DATE						•				30
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DATE TIME HOLE (FEET) (FEET) (FEET) WATER CONDITIONS OF OBSERVATION NO OBSERVATIONS MADE.	WATER LEVI	EL OBSER\	<u>/ATIONS IN E</u>	OREHOLE		· · · · · · · · · · · · · · · · · · ·		·····		
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CORE DRILLING IN ROCK LIN. FT. OTHER: BORING CONTRACTOR DRILLER RONALD STIDHAM HELPERS BOREHOLE WAS GROUTED UPON COMPLETION. RESIDENT ENGINEER MICHAEL WECKLER DATE 3-7-02										
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REMARKS BOREHOLE WAS GROUTED UPON COMPLETION. RESIDENT ENGINEER MICHAEL WECKLER DATE 3-7-02		THATON		D STIDHAM				BILLY LUT	TRELL	
RESIDENT ENGINEER MICHAEL WECKLER DATE 3-7-02										
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	BURING NO.	VV-Z	
	SHEET 1 OF	2	
ND IMPROVEMENTS	FILE NO.	9726A	
	CUDEACE ELEV	24.0	

PROJECT: LOCATION: WASHINGTON MONUMENT GROUN WASHINGTON, D.C.

DAILY PROCRESS NO. DEPTH BLOWS/6" SAMPLE DESCRIPTION STRATA DEPTH BLOWS REMARK	ace.
PROGRESS No. DEPTH BLOWS/6" SAMPLE DESCRIPTION STRATA DEPTH BLOWS REMARK	ace.
07:50 1D 0.0 4-6 Brown silty clay, sm f sand layers, tr brick, concrete, cinders, veg (Fill) (CL) Brown silty fine sand, some fine sandy clay layers, trace gravel, brick (Fill) (SM) Brown silty fine to medium sand, trace clay, gravel (Fill) (SM-SC) Stiff brown fine sandy silt, trace clay, gravel (ML) Brown fine sandy silt, trace clay (ML) T1(A) 8	
07.50	18
Thursday Clear	
Clear 4.0 4.9 layers, trace gravel, brick (Fill) (SM) F S WC=15	
3D 4.0 4-5 Brown silty fine to medium sand, trace clay, gravel (Fill) (SM-SC) 5 MC=15	
So T.0 6.0 6-8 4-5 Stiff brown fine sandy clay (CL) Stiff brown fine sandy clay (CL) Stiff brown fine sandy silt, trace clay, gravel (ML) So To To To To To To To To To To To To To	
4D 6.0	
S	
SD 8.0 10.0 10-11 6D 10.0 6-9 11-13	17
10.0 10-11 6D 10.0 6-9 11-13	117
6D 10.0 6-9 11-13 Brown fine sandy silt, trace clay (ML)	ļ
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12'. pp=Pocket 15 Penetrometel Unconfined C sive Strength WC=Water C	
12'. pp=Pocket 15 Penetrometer Unconfined C sive Strength WC=Water C	at
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WC=Water C	
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weight.	
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BORING NO. W-2	
BORING NO. W-2	

						BORING NO. SHEET	W-2 2 OF 2
PROJECT	/ / // CHI	NGTON MON	ILIMENT GRO	OUND IMPRO	/EMENTS	FILE NO.	9726A
LOCATION			GTON, D.C.	SONE IVII IVO		SURFACE EL	EV. 24.0
BORING LO			RING LOCAT	ION PLAN	 	DATUM	NGSD
SOKING E	JOANON	022 00.	<u> </u>				
30RING EQ	<u>UIPMENT A</u>	ND METHOD	S OF STABIL	IZING BOREL	OLE		
		—	OF FEED	0.4.0	NINO LICED	☐ VES	X NO
TYPE OF BO			IG CORING		SING USED	DEPTH, FT. FR	لنتا
TRUCK	CME-4				IN	DEPTH, FT. FR	
SKID		HYDRA			IN.	_	
BARGE OTHER		OTHER		DIA.,			
TYPE AND S	SIZE OF			DRILL	ING MUD USE	YES	X NO
D-SAMPLER		LIT SPOON		DIAME	TER OF ROTARY	BIT, IN.	
U-SAMPLER				TYPE C	F DRILLING MUD)	
S-SAMPLER		· · _					
				AUGE	R USED	X YES	NO
CORE BIT				TYPE A	ND DIAMETER, II	N	HOLLOW STEM 0.D. 6"
DRILL RODS	AW						
					HAMMER, LBS.		/ERAGE FALL, IN.
				SAMPL	ER HAMMER, LBS	s. <u>140</u> A\	/ERAGE FALL, IN. 30
MATERIEV	FLORSER\	ATIONS IN E	OREHOLE				
YVAI CIV CCY	LL ODOLI.	DEPTH OF	DEPTH OF	DEPTH TO			
DATE	TIME	HOLE	CASING	WATER	CON	NDITIONS OF OBSE	RVATION
		(FEET)	(FEET)	(FEET)			
					NO OBSERV	ATIONS MADE.	
							<u> </u>
PIEZOMETE	R INSTALLE	D YE	s X	NO SKET	CH SHOWN O	N	
OTANDDIDE:	TVDE			ID II	٧.	LENGTH, FT.	TOP ELEV.
STANDPIPE: INTAKE ELEM					v N		
		RIAL			N.		BOT. ELEV.
FILTER:	IVIATE				· · · _ · · · · · · · · · · · · · · · ·		
PAY QUANT	<u>ITIES</u>						
3.5" DIA. DRY	SAMPLE BOF	RING LIN.	FT			TUBE SAMPLES	
3.5" DIA. U-SA	MPLE BORIN	G LIN.	FT	N	O. OF 3" UNDIST	URBED SAMPLES	
CORE DRILLII	NG IN ROCK	LIN.	FT.	0	THER:		
	NEDACTOR				GEOSERVIC		· · · · · · · · · · · · · · · · · · ·
BOKING CO	NTRACTOR						
	NIRACIOR	RONAL	D STIDHAM		HELPERS		BILLY LUTTRELL
BORING CO DRILLER REMARKS		RONAL			HELPERS ON COMPLETION	ON.	
DRILLER		RONAL	HOLE WAS C		ON COMPLETI	ON.	ATE 3-7-02 BORING NO. W-2

PROJECT: LOCATION: WASHINGTON MONUMENT GROUND IMPROVEMENTS
WASHINGTON, D.C.

BORING NO. W-3

SHEET 1 OF 2

FILE NO. 9726A

SURFACE ELEV. 19.5

CALLY SAMPLE CALLY CAL							KES		WICHAEL VVECKLER
PROJECTION DEPTH BLOWNSF SAMPLE DESCRIPTION STRUTA EEPH ACCOUNTY CONTROL CONTR	DAILY		SAMPL	E				CASING	i
10		NO			SAMPLE DESCRIPTION	STRATA	DEPTH	BLOWS	REMARKS
1-9 1-9									Grass at surface.
Thursday 2D 2.0 4-6 Roys 3 4.0 5-8 Roys 3 4.0 6-4 Roys 3 4.0 6-4 Roys 3 4.0 6-4 Roys 3 4.0 6-4 Roys 3 4.0 6.0 4-5 8.0 9-11 Roys 3 8.0 Points 3 8.0 Poin	. 1	טו							
Click	03-07-02					_			10/0-17
Brown fine sandy slit, trace clay, clay pockets, gravel (FIM) (ML) Brown fine sandy slit, trace clay, clay pockets, gravel (FIM) (ML) Brown fine sandy slit, trace clay, clay pockets, gravel (FIM) (ML) Brown fine to medium sandy slit, trace clay, coarse sand, gravel (ML) Brown fine sandy slit, trace	Thursday	2D	2.0			F			VVC=17
807	Clear		4.0	5-8	(ML)				
S	I	3D	_		Brown fine sandy silt, trace clay, clay		5		WC=18
40 6.0 8.0 9-11 7-8 6 9-11 7-8 6 9-11 7-8 6 9-11 7-8 6 9-12 9-12 9-12 9-12 9-12 9-12 9-12 9-12	1 001	***************************************	<u> </u>	•		'	6		
S	,	-15			Prove fine to medium sandy silt trace clay				WC=15
SD 8.0 7-6 10.0 7-10 10 10 10 10 10 10 10		4D					0		1
10.0	, 1				coarse sand, graver (IVIL)	11 (A)	0		Į [
T2		5D	8.0	7-6	Brown silty fine sand, trace clay (SM-SC)				
SD 10.0 7-7 8-10 Do 5D, trace gravel (SM-SC) T2	i		10.0	7-10		ļ .	10	AND DESCRIPTION OF THE PARTY OF	
09:25 12:0 8-10 12 End of Boring at 12	1	6D	SAN TANKS AND ASSESSMENT OF THE PARTY OF THE		Do 5D, trace gravel (SM-SC)	T2			WC=15
12. WC=Water Content in percent of dry weight. 20. 30. 30. 30. 30. 31. 35. 35. 35. 35. 35. 35. 35. 35. 35. 35		00			,		12		End of Boring at
## WC=Water Content in percent of dry weight. 20	09:25		12.0	0-10			- · -		
15 in percent of dry weight. 20 25 30 40 45 45	J								-
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BORING NO. W-3				ļ	1				1
BORING NO. W-3	,							 	-
BORING NOVV-3							1-55	100 515	1/1/2
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						BORING NO)	W-3	
						SHEET _	2	OF _	2
PROJECT	WASH	INGTON MON	IUMENT GRO	OUND IMPRO	VEMENTS	FILE NO.	972	***************************************	
LOCATION	V V	WASHIN	GTON, D.C.			SURFACE E	ELEV	19.5	
BORING L	OCATION	SEE BOI	RING LOCAT	ON PLAN		DATUM _		NGSD	
		TYPE	OF FEED	IZING BOREL		□ vco		1 NO	
TYPE OF BOTRUCK	ORING RIG CME-4		IG CORING NICAL		SING USED IN	DEPTH, FT. F		NO -	го
SKID	OIVIL_1	HYDRA			IN.			-	го
BARGE		OTHER			IN.			-	ro
OTHER									
TYPE AND D-SAMPLER	2" O.D. SF	PLIT SPOON		DIAME	ING MUD USEE TER OF ROTARY OF DRILLING MUD	BIT, IN.	X	NO	
S-SAMPLER								_	
CORE BARRE	EL			AUGE	R USED	X YES		NO	
CORE BIT				TYPE A	ND DIAMETER, II	N	HOLL	OW STEM	O.D. 6"
DRILL RODS	AW								
WATER LE	VEL OBSER\	VATIONS IN E	OREHOLE		E HAMMER, LBS.		AVERAGE AVERAGE	FALL, IN	30
DATE	TIME	HOLE (FEET)	CASING (FEET)	WATER (FEET)	CON	NDITIONS OF OB	SERVATIO	N	
					NO OBSERV	ATIONS MADE	Ē		
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									<u>. </u>
	ER INSTALLI	<u>ED</u> YE	s X N		CH SHOWN O			OB ELEV	
STANDPIPE:					N			P ELEV.	
INTAKE ELEN					N	-		OT. ELEV.	
	SAMPLE BOF	RING LIN.		12 N	O. OF 3" SHELBY	TUBE SAMPLES			
	AMPLE BORIN				O. OF 3" UNDIST!	URBED SAMPLES			
CORE DRILLI	ING IN ROCK	LIN.	FT		THER:				
BORING CO	ONTRACTOR	₹			GEOSERVIC	ES CORP.			<u> </u>
DRILLER			D STIDHAM		HELPERS		BILLY L	JTTRELL	
REMARKS		BOR	EHOLE WAS	GROUTED U	PON COMPLE	TION.			
RESIDENT	ENGINEER			MICHAEL WE	CKLER		DATE	3-7-	-02
	•	<u></u>					BORI	NG NO.	W-3

BORING NO.	W-4	
SHEET 1 OF	2	
FILE NO.	9726A	
SUDEACE ELEV	14.6	

PROJECT: LOCATION: WASHINGTON MONUMENT GROUND IMPROVEMENTS
WASHINGTON, D.C.

LOCATIC	/IN .			WASHINGTON, D.O.		RES	. ENGR.	MICHAEL WECKLER
DAILY		SAMPLI	=				CASING	
PROGRESS	NO.	DEPTH	BLOWS/6"	SAMPLE DESCRIPTION	STRATA	DEPTH	BLOWS	REMARKS
09:35	1D	0.0	3-6	Brown silty fine sand, trace clay, gravel,			AUGER	Grass at surface.
03-07-02		2.0	6-8	clay layers, vegetation (Fill) (SM-SC)	F			
Thursday	2D	2.0	4-4	Brown silty fine sand, trace clay, brick (Fill)				
Clear		4.0	5-5	(SM-SC)		4		
60°F	3D	4.0	5-4	Brown silty fine sand, trace clay, clay layers	T2	5		
1 33.	***********	6.0	4-5	(SM-SC)		6		
	4D	6.0	4-3	Brown fine sandy silt, trace clay, fine sand	T1(A)			WC=16
		8.0	4-8	lenses (ML)		8		
	5D	8.0	6-5	Brown silty fine sand, trace clay, clay seams				
	-	10.0	4-5	(SM)	T2	10	A	
1	6D	10.0	4-3	Brown fine sand, some silt, trace clay	·			End of Boring at
10:30		12.0	3-4	pockets (SM)		12		12'.
10.55								
				·				WC=Water Content
				·		15		in percent of dry
1	***********							weight.
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	l	ـــــا				BOR	ING NO	. W-4

						BORING NO	·	W-4	
						SHEET	2	OF	2
PROJECT	WASHI	NGTON MON	UMENT GRO	OUND IMPRO	VEMENTS	FILE NO.		'26A	
OCATION			GTON, D.C.	. <u> </u>		SURFACE E	LEV	14.0	5
BORING LO	CATION	SEE BOF	RING LOCAT	ION PLAN		DATUM _	-	NGSD	
TYPE OF BO TRUCK SKID BARGE OTHER TYPE AND S D-SAMPLER J-SAMPLER S-SAMPLER	ORING RIG CME-45 CIZE OF: 2" O.D. SF	TYPE DURIN	OF FEED IG CORING NICAL ULIC	DIA. DIA. DIA. DRILL DIAME TYPE C	HOLE SING USED , IN. , IN. , IN. ING MUD USED TER OF ROTARY OF DRILLING MUD R USED AND DIAMETER, IF	DEPTH, FT. F DEPTH, FT. F DEPTH, FT. F DEPTH, FT. F DEPTH, FT. F	ROM ROM		TO TO TO
	EL OBSERV	ATIONS IN E	OREHOLE DEPTH OF CASING		G HAMMER, LBS. ER HAMMER, LBS		AVERAG	E FALL, IN. E FALL, IN.	30
DATE	TIME	(FEET)	(FEET)	(FEET)					
					NO OBSERV	ATIONS MADE			···································
							<u> </u>		
STANDPIPE: NTAKE ELEMI	TYPE ENT: TYPE			ID, I	rch shown of	LENGTH, FT LENGTH, FT		TIP ELEV.	
TANDPIPE: NTAKE ELEMI	TYPE ENT: TYPE MATER			ID, I	N IN	LENGTH, FT LENGTH, FT		TIP ELEV.	
TANDPIPE: NTAKE ELEMI ILTER: 'AY QUANTI	TYPE ENT: TYPE MATER	RIAL		ID, I OD, OD,	N IN O. OF 3" SHELBY	LENGTH, FT. LENGTH, FT. LENGTH, FT. TUBE SAMPLES		TIP ELEV.	
TANDPIPE: NTAKE ELEM! ILTER: PAY QUANTI .5" DIA. DRY S	TYPE ENT: TYPE MATER ITIES SAMPLE BOR	RIAL LIN.	FT	ID, I OD, OD,	N IN IN	LENGTH, FT. LENGTH, FT. LENGTH, FT. TUBE SAMPLES		TIP ELEV.	
TANDPIPE: NTAKE ELEM! ILTER: PAY QUANT! .5" DIA. DRY :	TYPE ENT: TYPE MATER ITIES SAMPLE BORING	RIAL LIN.	FT	ID, I OD, OD, 12 N	N IN O. OF 3" SHELBY	LENGTH, FT. LENGTH, FT. LENGTH, FT. TUBE SAMPLES		TIP ELEV.	
STANDPIPE: NTAKE ELEMI ILTER: PAY QUANTI .5" DIA. DRY S .5" DIA. U-SAI CORE DRILLIN	TYPE ENT: TYPE MATER ITIES SAMPLE BOR MPLE BORING NG IN ROCK	RIAL LIN.	FT FT	ID, I OD, OD, 12 N N	N. IN. O. OF 3" SHELBY O. OF 3" UNDISTO THER:	LENGTH, FT. LENGTH, FT. LENGTH, FT. TUBE SAMPLES JRBED SAMPLES	5 _	TIP ELEV.	
TANDPIPE: NTAKE ELEM! ILTER: PAY QUANTI .5" DIA. DRY S .5" DIA. U-SAI CORE DRILLIN	TYPE ENT: TYPE MATER ITIES SAMPLE BOR MPLE BORING NG IN ROCK	RIAL LIN. G LIN. LIN.	FTFT	ID, I OD, OD, 12 N N O	N. IN. O. OF 3" SHELBY O. OF 3" UNDISTO THER: GEOSERVIC HELPERS	LENGTH, FT. LENGTH, FT. LENGTH, FT. TUBE SAMPLES JRBED SAMPLES	5 _	TIP ELEV.	
PIEZOMETEI STANDPIPE: NTAKE ELEMI FILTER: PAY QUANTI 3.5" DIA. DRY S 3.5" DIA. U-SAI CORE DRILLIN BORING COI DRILLER REMARKS RESIDENT E	TYPE ENT: TYPE MATER ITIES SAMPLE BORN MPLE BORN NG IN ROCK NTRACTOR	RIAL LIN. G LIN. LIN.	FTFT	ID, I OD, OD, 12 N N O	N. IN. O. OF 3" SHELBY O. OF 3" UNDISTO THER: GEOSERVIC HELPERS ON COMPLETO	LENGTH, FT. LENGTH, FT. LENGTH, FT. TUBE SAMPLES JRBED SAMPLES	BILLY	TIP ELEV. BOT. ELEV. LUTTRELL	

PROJECT:	
LOCATION	•

WASHINGTON MONUMENT GROUND IMPROVEMENTS

WASHINGTON, D.C.

BORING NO. SHEET 1 OF _____ 9726A 15.4 SURFACE ELEV.

						1	CASING	
DAILY		SAMPLI		SAMPLE DESCRIPTION	STRATA	DEPTH	BLOWS	REMARKS
PROGRESS	NO.	DEPTH	BLOWS/6"	Brn silty fine sand, sm fine sandy clay				Grass at surface.
10:30	_1D_	0.0	3-8	pockets, tr clay, brick, gvl (Fill) (SM-SC)		<u> </u>		
03-07-02		2.0	8-9	Brown fine sandy clay, tr cinders (Fill) (CL)				
Thursday	2D	2.0	3-6	Brown fine sandy day, it diliders (i iii) (OL)			 - -	
Clear		4.0_	8-9			5	-	pp=3.5, WC=20
60°F	3D	4.0	4-27	Brown silty clay, some fine sand, trace			************	pp-0.0, 110 20
		6.0	12-9	gravel, roots (Fill) (CL)				
	4D	6.0	4-5	Brown silty f sand, trace clay, clay pockets,				-
		8.0	6-6	brick, lignite (Fill) (SM)	F			
	5D	8.0	3-4	Top: Brn si f sand, sm cl pkts, brk (Fill) (SM)		<u> </u>		
		10.0	5-6	Bot 6': Brn f-m sand, sm silt (Fill) (SM)		10		
1	6D	10.0	4-3	Brown silty fine sand, trace clay pockets,				Sample moist at
		12.0	3-5	brick, gravel (Fill) (SM)		ļ <u>.</u>	<u> </u>	bottom 6".
	7D	12.0	3-2	Brown silty fine sand, some fine sandy clay				End of Boring at
	70	14.0	3-3	pockets, trace brick, gravel (Fill) (SM&CL)	İ		<u> </u>	12'.
	8D	14.0	2-2	Brown fine to medium sand, some silt, trace		15	***************************************	
		16.0	4-3	brick, clay pockets, gravel (Fill) (SM)		16		pp=Pocket
11:15		10.0	7-5					Penetrometer
		 						Unconfined Compres-
		 	-					sive Strength in tsf.
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						BORING NO.	W-5		
						SHEET	2 OF 2		
PROJECT	WASHI	NGTON MON	IUMENT GRO	OUND IMPRO	/EMENTS	FILE NO.	9726A		
LOCATION			GTON, D.C.			SURFACE ELE			
BORING LO		SEE BOR	RING LOCAT	ION PLAN		DATUM	NGSD		
BORING EQU TYPE OF BOI TRUCK	·	TYPE DURIN	OF FEED IG CORING		HOLE SING USED IN.	YES DEPTH, FT. FRO	X NO TO		
SKID		 HYDRA	ULIC		IN		м то		
BARGE		OTHER			IN		м то		
OTHER TYPE AND SI D-SAMPLER		LIT SPOON			ING MUD USE		X NO		
U-SAMPLER				TYPE C	F DRILLING MU	D			
S-SAMPLER CORE BARREL CORE BIT DRILL RODS			···· -		R USED ND DIAMETER,		NO HOLLOW STEM O.D. 6"		
					S HAMMER, LBS ER HAMMER, LE		ERAGE FALL, IN		
WATER LEVE	L OBSERV	ATIONS IN E	OREHOLE						
DATE	TIME	DEPTH OF HOLE (FEET)	DEPTH OF CASING (FEET)	DEPTH TO WATER (FEET)	cc	NDITIONS OF OBSER	RVATION		
					NO OBSER	VATIONS MADE.			
							<u> </u>		
		· · · · · · · · · · · · · · · · · · ·							
		<u> </u>							
PIEZOMETER STANDPIPE:				10.11	CH SHOWN C		TOP ELEV		
INTAKE ELEME					N		TIP ELEV.		
FILTER:	MATER	IAL			N		BOT. ELEV.		
PAY QUANTI 3.5" DIA. DRY S 3.5" DIA. U-SAN CORE DRILLING	<u>TIES</u> SAMPLE BOR MPLE BORING	ING LIN.	FT	16 N	D. OF 3" SHELB	Y TUBE SAMPLES FURBED SAMPLES			
BORING CON	ITRACTOR				GEOSERVIO	CES CORP.	<u></u>		
DRILLER	BORING CONTRACTOR RONALD STIDHAM					PERS BILLY LUTTRELL			
REMARKS			BORE	HOLE WAS G	ROUTED UPO	ON COMPLETION.			
RESIDENT E	NGINEER			MICHAEL WE	CKLER	DA	ATE 3-7-02		
							BORING NO. W-5		

PROJECT: LOCATION: WASHINGTON MONUMENT GROUND IMPROVEMENTS

WASHINGTON, D.C.

BORING NO. W-6
SHEET 1 OF 2
FILE NO. 9726A

SURFACE ELEV. 21.9

						RES	. ENGR. MICHAEL WECKLE	<u> </u>
DAILY		SAMPL	E				CASING	
PROGRESS	NO.	DEPTH	BLOWS/6"	SAMPLE DESCRIPTION	STRATA	DEPTH		-
06:30	1D	0.0	7-10	Brown silty f-c sand, some gravel, trace			AUGER Grass at surface.	
03-07-02		2.0	26-14	clay, vegetation (Fill) (SM)			+ + - 1	
Thursday	2D	2.0	15-16	Brown gravelly f-c sand, some silt, trace			 	
Clear		4.0	14-17	clay pkts, brick (Fill) (SM) Brown fine to coarse sand, some gravel,		5	 - - 	
60°F	3D	4.0	14-8	silt, trace cinders (Fill) (SM)	F		10000000000000000000000000000000000000	
	45	6.0	4-6 5-4	Int brn f-m sand, sm silt, gvl, tr clay pkts,	•			
	4D	6.0 8.0	5- 4 5-8	brk & blk f sandy silt, tr cl (Fill) (SM&ML)				
	5D	8.0	5-6	Brown silty fine sand, some clay pockets,				
	30	10.0	7-9	trace gravel (Fill) (SM)		10	₩	
	6D	10.0	7-6	Brown silty fine sand, some clay layers,		100000000000000000000000000000000000000	End of Boring at	
07:00	-05	12.0	7-8	trace gravel, clay (Fill) (SM-SC)		12	12'.	
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L	L	L	<u> </u>			BOR	ING NO W-6	

				BORING NO.				
				SHEET _	OF	2		
PROJECT WASHINGTON M	ONUMENT GR	OUND IMPROVE	MENTS	FILE NO.	9726A			
LOCATION WASH		SURFACE EL						
BORING LOCATIONSEE E	ORING LOCAT	TION PLAN		DATUM	NGSD			
BORING EQUIPMENT AND METH	ODS OF STABI	LIZING BOREHÇ	<u>DLE</u>					
TYF	E OF FEED							
TYPE OF BORING RIG DUI	RING CORING	CASI	NG USED	YES	X NO			
TRUCK CME-45 MEC	HANICAL		۱					
SKID HYD	RAULIC		۱					
BARGEOTH	ER	DIA., IN	DIA., IN DEPTH, FT. FROM			то		
OTHER								
TYPE AND SIZE OF		DRILLIN	G MUD USED	YES	X NO			
TYPE AND SIZE OF: D-SAMPLER 2" O.D. SPLIT SPOC	N		R OF ROTARY I	<u> </u>	۰۰۰ ست			
			DRILLING MUD					
U-SAMPLER		111 2 01	DI NICENTO IN O					
S-SAMPLER CORE BARREL		AUGER	USED	X YES	NO			
CORE BIT			D DIAMETER, IN		HOLLOW STE	M O.D. 6"		
DRILL RODS AW								
DRILL RODS AVV		CASING H	HAMMER, LBS.	A	VERAGE FALL, IN.	•		
			R HAMMER, LBS		VERAGE FALL, IN.			
AND THE SPECIAL ONE II	L DODENOLE							
WATER LEVEL OBSERVATIONS II								
DATE TIME HOLE (FEET)	HOLE CASING WATER							
			NO OBSERV	ATIONS MADE.				
PIEZOMETER INSTALLED	res X i	NO SKETC	H SHOWN ON	,				
STANDPIPE: TYPE		ID IN		LENGTH, FT.	TOP ELEV.			
STANDPIPE: TYPE INTAKE ELEMENT: TYPE				LENGTH, FT.	TIP ELEV.			
FILTER: MATERIAL		 		LENGTH, FT.	BOT. ELEV.	 -		
FILTER. WATERIAL								
PAY QUANTITIES								
3.5" DIA. DRY SAMPLE BORING L	IN. FT			TUBE SAMPLES		<u> </u>		
3.5" DIA. U-SAMPLE BORING L	OF 3" UNDISTU	JRBED SAMPLES						
CORE DRILLING IN ROCK	IN. FT		IER:					
BORING CONTRACTOR			GEOSERVICE					
DRILLER RON	IALD STIDHAM	l	HELPERS		BILLY LUTTRELL			
REMARKS BOF	EHOLE WAS	GROUTED UPON	N COMPLETIC					
RESIDENT ENGINEER		MICHAEL WEC	KLER	[8-02		
					BORING NO.	W-6		

PROJECT: LOCATION: WASHINGTON MONUMENT GROUND IMPROVEMENTS
WASHINGTON, D.C.

BORING NO. W-7U

SHEET 1 OF 2

FILE NO. 9726A

SURFACE ELEV. 21.0

								MICHAEL WECKLER
DAILY	LY SAMPLE						CASING	
i i				SAMPLE DESCRIPTION	STRATA	DEPTH	BLOWS	REMARKS
PROGRESS	NO.	DEPTH	BLOWS/6"	Brown f-c sand, some silt, gravel, trace	011111111	<u> </u>		Grass at surface.
07:10	1 <u>D</u>	0.0	4-12	·			70001	diass at suriass.
03-07-02		2.0	18-22	brick, vegetation (Fill) (SM)				
Thursday	2D	2.0	8-11	Brn f-m sand, sm gravel, silt, fine sandy clay		ļ	<u> </u>	
Clear		4.0	10-14	pockets, trace clay (Fill) (SM-SC)				
_ 60°E	3D	4.0	7-8	Brown f-m sand, sm cinders, silt, trace		5_		
60	<u> </u>	6.0	8-12	brick, gravel, mica, clay pkts (Fill) (SM)	F			
	45		7-9	Brown red brn f-m sand, sm gravel, silt, tr				
_	4D	6.0	1	cinders, brick, clay (Fill) (SM)			 	
_		8.0	10-18				-	
	5D	8.0	11-15	Brn red brn silty f-c sand, sm gravel, clay				l
		10.0	14-9	pockets, trace brick, cinders (Fill) (SM)		10		1U Top pp=3.0
1	1U	10.0	PUSH=24"	Top: Brn f-c sandy silt, some gravel, trace		11		WC=13
08:00		12.0	REC=19"	clay (Fill) (ML)	T1A	12		1U Bot pp=2.0
08.00		12.0	1,20=.0	Bot: Stiff brn clayey silt, sm f sand, tr				TV=0.7, WC=21
		<u> </u>		gravel (ML)		<u> </u>		
				graver (IVIL)		4.5		End of Poring at 10
						15	<u> </u>	End of Boring at 12'.
j					ļ			pp=Pocket
- .					1			Penetrometer
								Unconfined Compres-
								sive Strength in tsf.
		<u> </u>				20	····	1 [
-								TV=Torvane Shear
)					İ		 	-1
						ļ		Strength in tsf.
						ļ	-	.
_								WC=Water Content
_						25	<u> </u>	in percent of dry
•				·	i			weight.
		-						1 1
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-						50		<u> </u>
			1					
	L	L		1		BOR	ING NO	. W-7U

						BORING NO		W-7U	
						SHEET _	2	OF	2
PROJECT	WASH	INGTON MON	FILE NO		′26A				
LOCATION	V	WASHIN	GTON, D.C.		SURFACE ELEV. 21.0 DATUM NGSD				
BORING L	BORING LOCATION SEE BORING LOCATION PLAN							NGSD	
BORING EC	QUIPMENT A			IZING BORE	HOLE				
TYPE OF B	ORING RIG		OF FEED IG CORING	CAS	SING USED	YES	X	NO	
TRUCK	CME-4	5 MECHA	NICAL		, IN		_		то
SKID		HYDRA	ULIC		, IN	_ DEPTH, FT. F			то
BARGE		OTHER		DIA.	, IN	_ DEPTH, FT. F	ROM		то
OTHER									
TYPE AND	SIZE OF:			DRILL	ING MUD USED	YES		X NO	
D-SAMPLER	2" O.D. SF	PLIT SPOON		DIAME.	TER OF ROTARY	BIT, IN			
U-SAMPLER	3" SHELB	Y HEAD		TYPE C	OF DRILLING MUD				
S-SAMPLER								_	
CORE BARRE	EL			AUGE	R USED	X YES		NO	
CORE BIT				TYPE A	ND DIAMETER, IN	1	HOLL	OW STEM	O.D. 7.5"
DRILL RODS	AW								
					3 HAMMER, LBS.			FALL, IN.	
				SAMPL	ER HAMMER, LBS	s. <u>140</u> <i>A</i>	AVERAGE	FALL, IN.	30
WATER LEV	/EL OBSER\	ATIONS IN E	<u>OREHOLE</u>						
		DEPTH OF	DEPTH OF	DEPTH TO					
DATE	TIME	HOLE (FEET)	CASING (FEET)	WATER (FEET)	CON	IDITIONS OF OBS	SERVATIO	ON	
		(,	(1221)	(1227)	NO OBSERV	ATIONS MADE			
					140 OBOLIKY	7110110 11710	·		
						-			
PIEZOMETE	ER INSTALLE	D YE	s X	IO SKET	CH SHOWN ON	١			·····
STANDPIPE:	TYPE			וו חו	N	LENGTH, FT.	7	TOP ELEV.	
INTAKE ELEM					N.			ΓΙΡ E LE V.	
FILTER:	MATER				N.			OT. ELEV.	
I ILI LIV.	100 (12)					_ · -			<u> </u>
PAY QUAN	<u>TITIES</u>								
3.5" DIA. DRY	SAMPLE BOR	RING LIN.	FT	10 N	O. OF 3" SHELBY	TUBE SAMPLES	_		
3.5" DIA. U-SAMPLE BORING LIN. FT. 2 NO. OF 3" UNDIS						JRBED SAMPLES		1	
CORE DRILLI	ING IN ROCK	LIN.	FT	O	THER:				
					•				
BORING CO	NTRACTOR				GEOSERVICE	ES CORP.			
DRILLER	DOWN DOTTOLIAM						BILLY L	UTTRELL	
REMARKS			ON COMPLETIC	DN.					
RESIDENT I	ENGINEFR			MICHAEL WE			DATE _	3-8	-02
							BOR	ING NO.	W-7U